

TOWN OF NORTH KINGSTOWN, RHODE ISLAND WICKFORD WATERFRONT IMPROVEMENTS

CONSTRUCTION DOCUMENTS APRIL 2024

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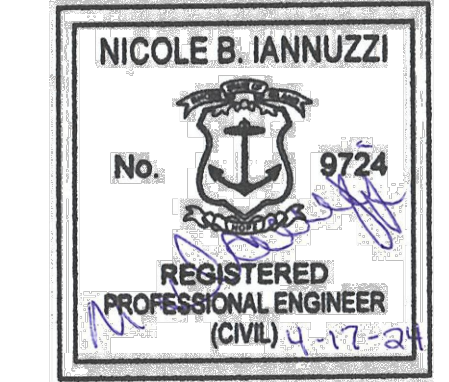
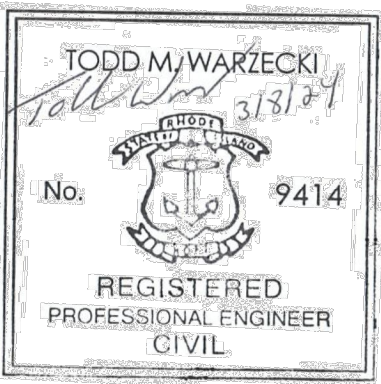
TOWN OF NORTH KINGSTOWN, RI
100 FAIRWAY DRIVE, NORTH KINGSTOWN, RI 02852

TOWN MANAGER
RALPH MOLLIS



LOCATION MAP
1" = 250'-0"

PREPARED BY:



REGISTERED PROFESSIONAL

4-17-2024
DATE

GENERAL SYMBOLS

PROPOSED

EXISTING

PROPOSED

ABBREVIATIONS

GENERAL

UTILITIES

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SUBCONSULTANT

PROJECT

Wickford Waterfront Improvements

Wickford, RI

TITLE

LEGEND & ABBREVIATIONS

NO.	REVISIONS	DATE

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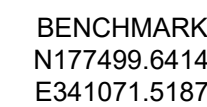
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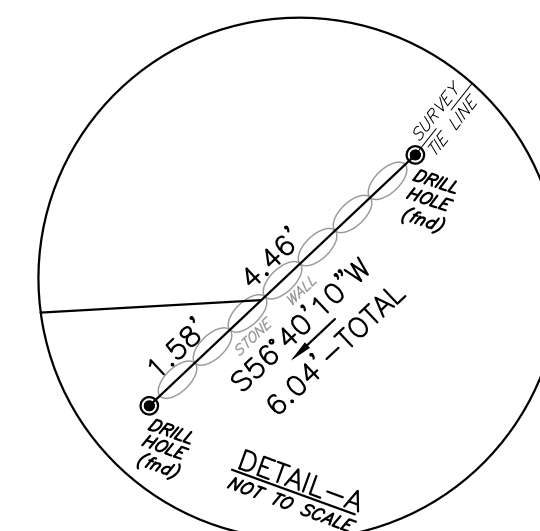
SHEET NO

2



FRANKLIN STREET
(PUBLIC~30' WIDE)

GRANITE BOUND
w/ DRILL HOLE
(fnd)(buried)

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SHEET NO.

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FRANKLIN STREET
(PUBLIC~30' WIDE)

TEMP CONSTRUCTION GATE
TEMP CONSTRUCTION ACCESS

PROPOSED INLET SEDIMENT CONTROL DEVICE (TYP.)

BROWN STREET
(PUBLIC~62' WIDE~1966 LAYOUT)

PROPOSED INLET SEDIMENT CONTROL DEVICE (TYP.)

AP 117, LOT 212
N/F
WALGREEN EASTERN CO. INC.
C/O WALGREEN CO.
BOOK 3249, PAGE 65

AP 117, LOT 211
N/F
OLD LIBRARY PROJECT LLC
BOOK 3323, PAGE 174

AP 117, LOT 209
N/F
SHIELDS FAMILY TRUST
BOOK 2489, PAGE 1

AP 117, LOT 204
N/F
35 BROWN, LLC
BOOK 2694, PAGE 23

AP 117, LOT 207
N/F
WICKFORD LANDINGS LLC
BOOK 2624, PAGE 13

WICKFORD HARBOR

R&R UTILITY POLE (BY OTHERS)

PROPOSED COFERDAM

TEMP. CONSTRUCTION FENCE (TYP.)

PROPOSED INLET SEDIMENT CONTROL DEVICE (TYP.)

PROPOSED INLET SEDIMENT CONTROL DEVICE (TYP.)

TEMP. CONSTRUCTION FENCE (TYP.)

TEMP. CONSTRUCTION FENCE (TYP.)

PROPOSED INLET SEDIMENT CONTROL DEVICE (TYP.)

PROPOSED COMPOST FILTER SOCK (TYP.)

PROPOSED INLET SEDIMENT CONTROL DEVICE (TYP.)

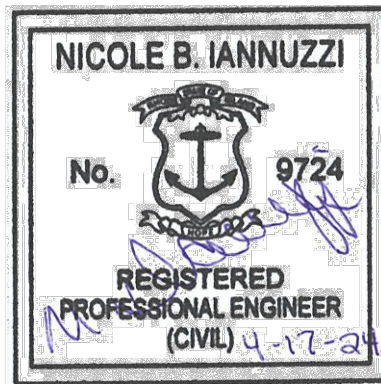
WICKFORD COVE

PROP. TREE PROTECTION

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PROJECT

Wickford Waterfront Improvements

Wickford, RI

TITLE

SITE PREPARATION PLAN

NO.	REVISIONS	DATE
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5



FRANKLIN STREET
(PUBLIC~30' WIDE)

BROWN STREET
(PUBLIC~62' WIDE~1956 LAYOUT)

WICKFORD HARBOR

WICKFORD COVE

GRANITE BOUND
w/ DRILL HOLE
(Ind/Burned)

AP 117, LOT 204
N/F
35 BROWN, LLC
BOOK 2694, PAGE 23

AP 117, LOT 207
N/F
WICKFORD LANDINGS LLC
BOOK 2624, PAGE 13

PHASE 3

STAGING AREA

PHASE 1

PHASE 2

SUGGESTED CONSTRUCTION SEQUENCING:
CONTRACTOR SHALL FOLLOW THE CONSTRUCTION SEQUENCING LISTED BELOW:

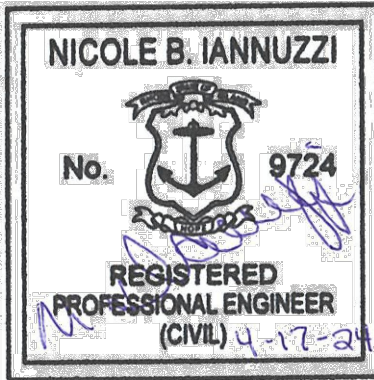
- INSTALLATION OF COFFERDAM, FOOTINGS AND PILES FOR THE PROPOSED WALL MUST BE COMPLETE PRIOR TO ANY OTHER WORK
- FINISH CONSTRUCTION OF PROPOSED WALL AND INSTALL DRAINAGE INFRASTRUCTURE
- FURNISH AND INSTALL CURBING, INCLUDING 4" CURB EDGING TO PERMEABLE PAVER AREAS AND CURB ASSOCIATED WITH PROPOSED BMP
- REMOVE AND DISPOSE EXISTING PAVEMENT, GRADING OF PARKING LOT, INSTALL CLASS 19.0 HMA BASE COURSE AND TEMPORARY PAVEMENT MARKINGS IN ACCORDANCE WITH THE PHASES LISTED BELOW.
 1. PHASE 1
 - ENTRANCE: A
 - EXIT: B
 2. PHASE 2
 - ENTRANCE: A
 - EXIT: C
 3. PHASE 3
 - ENTRANCE: B
 - EXIT: C
- UPON COMPLETION OF THE PROPOSED WALL, INSTALL PERMEABLE PAVERS, CLASS 9.5 HMA SURFACE COURSE, PERMANENT PAVEMENT MARKINGS AND FINAL SITE RESTORATION

CONTRACTOR SHALL PROVIDE ACCESS TO A ENTRANCE AND EXIT FROM THE PARKING SITE AT ALL TIMES DURING CONSTRUCTION. TRAFFIC FLOW SHALL NOT OCCUR ON THE PERMEABLE PAVERS DURING PHASING.

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PHASING PLAN

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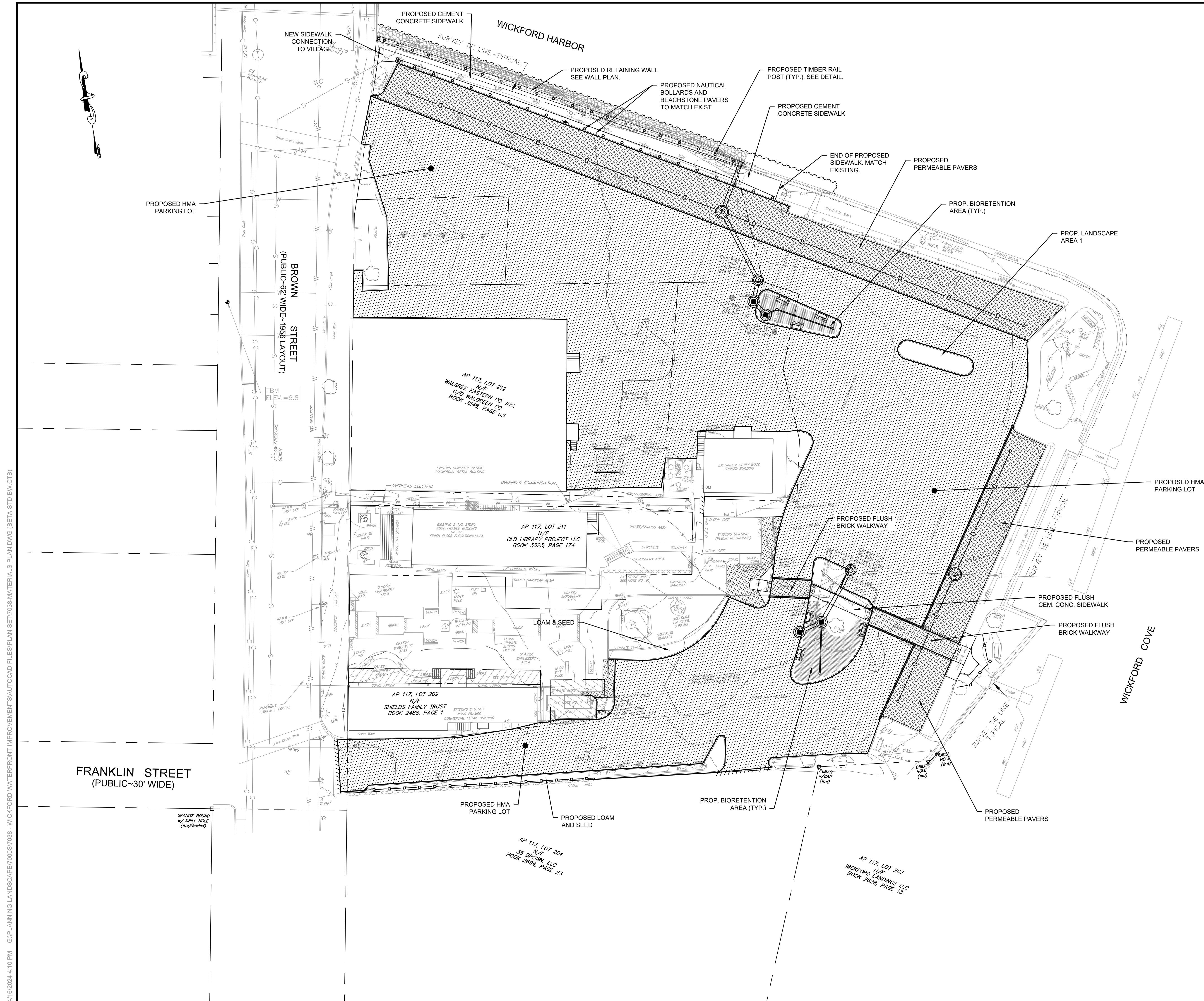
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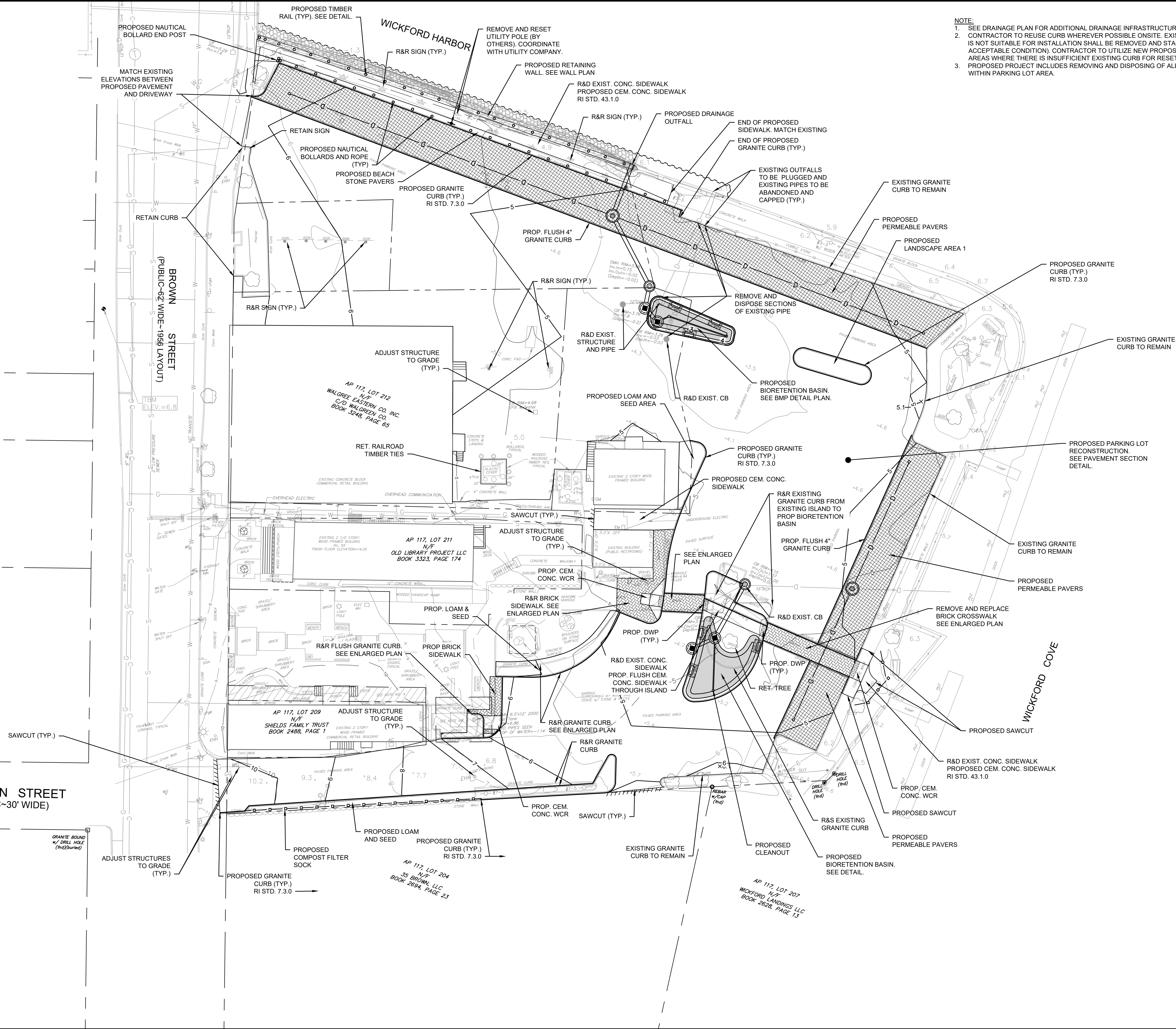
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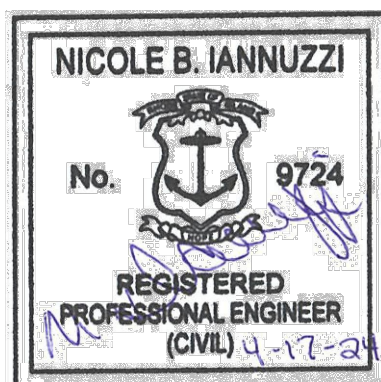


- NOTE:
1. SEE DRAINAGE PLAN FOR ADDITIONAL DRAINAGE INFRASTRUCTURE INFORMATION.
 2. CONTRACTOR TO REUSE CURB WHEREVER POSSIBLE ONSITE. EXISTING CURB THAT IS NOT SUITABLE FOR INSTALLATION SHALL BE REMOVED AND STACKED (IF IN ACCEPTABLE CONDITION). CONTRACTOR TO UTILIZE NEW PROPOSED CURB IN AREAS WHERE THERE IS INSUFFICIENT EXISTING CURB FOR RESETTING.
 3. PROPOSED PROJECT INCLUDES REMOVING AND DISPOSING OF ALL PAVEMENT WITHIN PARKING LOT AREA.

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**Wickford Waterfront
Improvements**

Wickford, RI

TITLE

SITE PLAN

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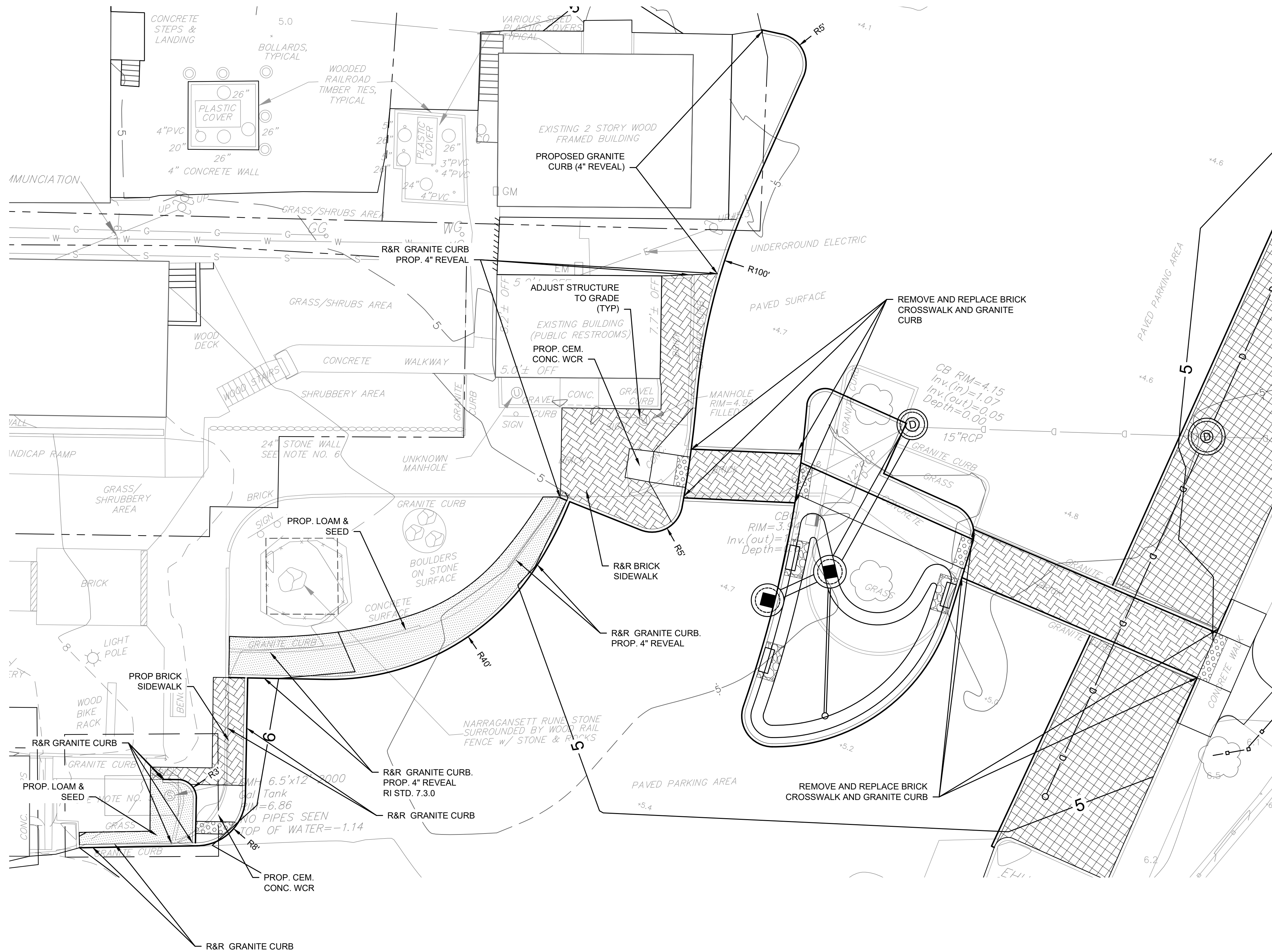


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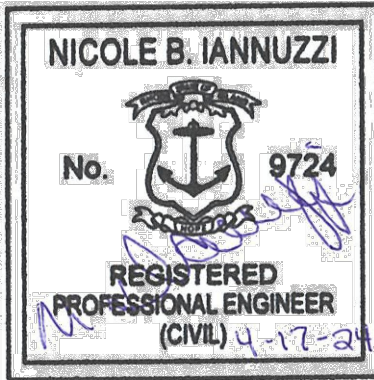


NOTE:
1. CONTRACTOR TO REUSE CURB WHEREVER POSSIBLE ONSITE. EXISTING CURB THAT IS NOT SUITABLE FOR INSTALLATION SHALL BE REMOVED AND STACKED (IF IN ACCEPTABLE CONDITION). CONTRACTOR TO UTILIZE NEW PROPOSED CURB IN AREAS WHERE THERE IS INSUFFICIENT EXISTING CURB FOR RESETTNG.

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PROJECT

Wickford Waterfront
Improvements

Wickford, RI

TITLE

ENLARGED PLAN

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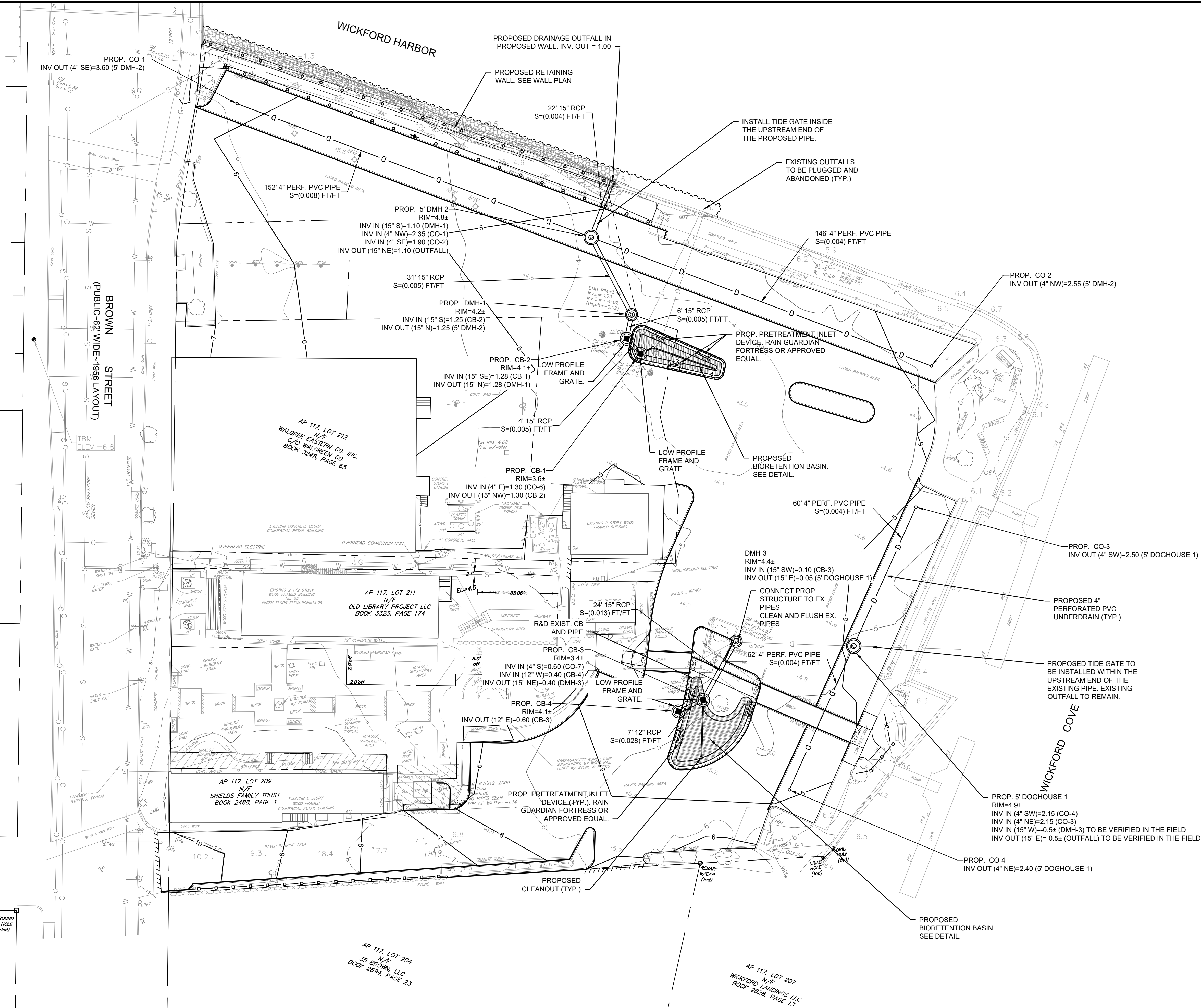
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FRANKLIN STREET
(PUBLIC~30' WIDE)

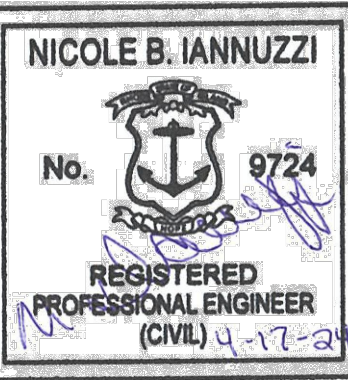
DRAINAGE NOTES:
1. ALL EXISTING AND PROPOSED PIPES WITHIN THE PROJECT LIMITS ARE TO BE CLEANED AND FLUSHED AT THE END OF THE PROJECT AND PRIOR TO PROJECT CLOSEOUT.



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Wickford Waterfront
Improvements

Wickford, RI

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DRAINAGE
PLAN

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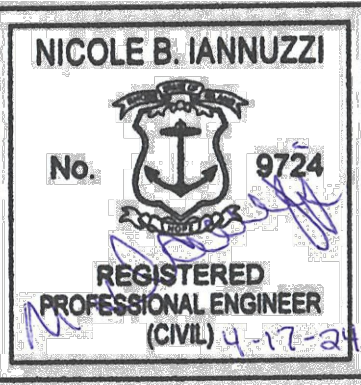
- NOTES:
1. GRADES SHOWN ARE BOTTOM OF CURB UNLESS OTHERWISE NOTED.
 2. REVEAL AT PROPOSED CURB LOCATIONS IS 6" UNLESS OTHERWISE NOTED.

Curve Table					
Curve #	Radius	Length	Chord Direction	Start Point	End Point
C1	5.000	7.223	S 78° 42' 49" W	(N:177583.3474,E:341159.5928)	(N:177582.0536,E:341153.1098)
C2	4.000	7.052	N 72° 46' 04" E	(N:177459.7664,E:341312.5263)	(N:177461.5954,E:341318.4229)
C3	5.000	7.854	N 22° 44' 27" W	(N:177447.5096,E:341312.9122)	(N:177454.0310,E:341310.1788)
C4	3.000	4.712	S 67° 15' 33" W	(N:177437.0282,E:341346.4404)	(N:177435.3882,E:341342.5276)
C5	3.000	4.136	S 17° 13' 56" E	(N:177443.2557,E:341346.3672)	(N:177439.6113,E:341347.4976)
C6	4.448	13.974	N 26° 20' 27" E	(N:177419.8595,E:341373.7092)	(N:177427.8320,E:341377.6565)
C7	4.448	13.974	S 26° 20' 27" W	(N:177415.8057,E:341401.9461)	(N:177407.8332,E:341397.9987)
C8	5.000	8.894	S 16° 47' 03" E	(N:177394.9404,E:341322.5527)	(N:177387.5046,E:341324.7955)
C9	100.000	33.730	S 24° 30' 34" W	(N:177369.3496,E:341312.4701)	(N:177338.8041,E:341298.5436)
C10	182.000	17.828	S 17° 39' 10" W	(N:177338.8041,E:341298.5436)	(N:177321.8221,E:341293.1393)
C11	5.000	8.842	S 71° 07' 21" W	(N:177321.8221,E:341293.1393)	(N:177319.3197,E:341285.8210)
C12	62.000	12.197	S 37° 25' 17" W	(N:177325.8716,E:341275.2480)	(N:177316.2007,E:341267.8482)
C13	40.000	40.514	S 72° 04' 23" W	(N:177316.2007,E:341267.8482)	(N:177304.2566,E:341230.9280)
C14	8.000	12.282	S 55° 04' 10" W	(N:177287.5080,E:341214.7870)	(N:177281.1462,E:341205.6780)
C15	3.000	4.743	N 34° 12' 29" W	(N:177288.8000,E:341206.8880)	(N:177292.3268,E:341204.4905)
C16	3.000	5.447	N 72° 09' 11" E	(N:177334.8750,E:341316.1224)	(N:177336.3244,E:341320.6242)
C17	200.000	50.514	N 21° 52' 48" E	(N:177288.1238,E:341297.3477)	(N:177334.8750,E:341316.1224)
C18	5.000	9.471	N 25° 09' 03" W	(N:177280.7760,E:341300.7976)	(N:177288.1238,E:341297.3477)
C19	30.000	38.532	S 63° 47' 17" W	(N:177296.6491,E:341333.0392)	(N:177280.7760,E:341300.7976)
C20	5.000	7.227	S 14° 25' 02" E	(N:177314.2608,E:341337.1001)	(N:177307.8546,E:341338.7470)
C21	2.000	5.306	S 64° 34' 56" E	(N:177263.2796,E:341258.6424)	(N:177261.6138,E:341262.1478)
C22	5.000	7.451	S 54° 06' 34" W	(N:177251.5456,E:341260.1148)	(N:177247.5704,E:341254.6214)

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PROJECT

Wickford Waterfront Improvements

Wickford, RI

TITLE

CURB, TIE & GRADING PLAN

NO.	REVISIONS	DATE
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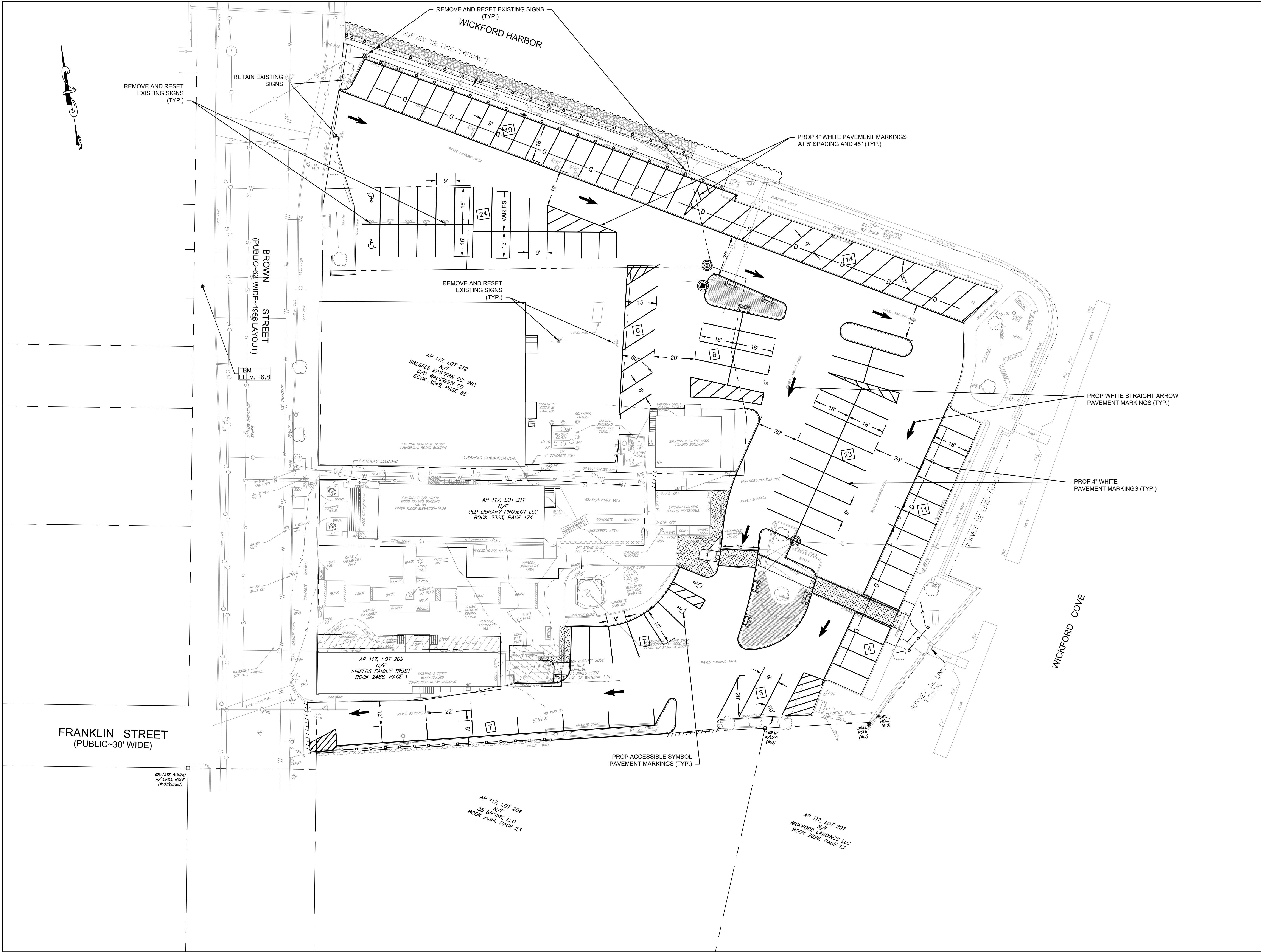
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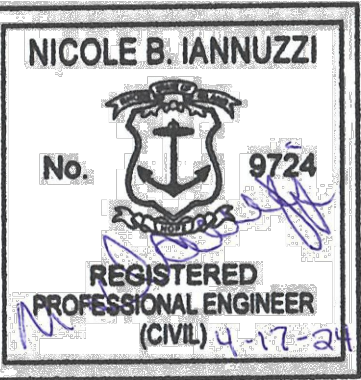
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Wickford Waterfront Improvements

Wickford, RI

TITLE

STRIPING PLAN

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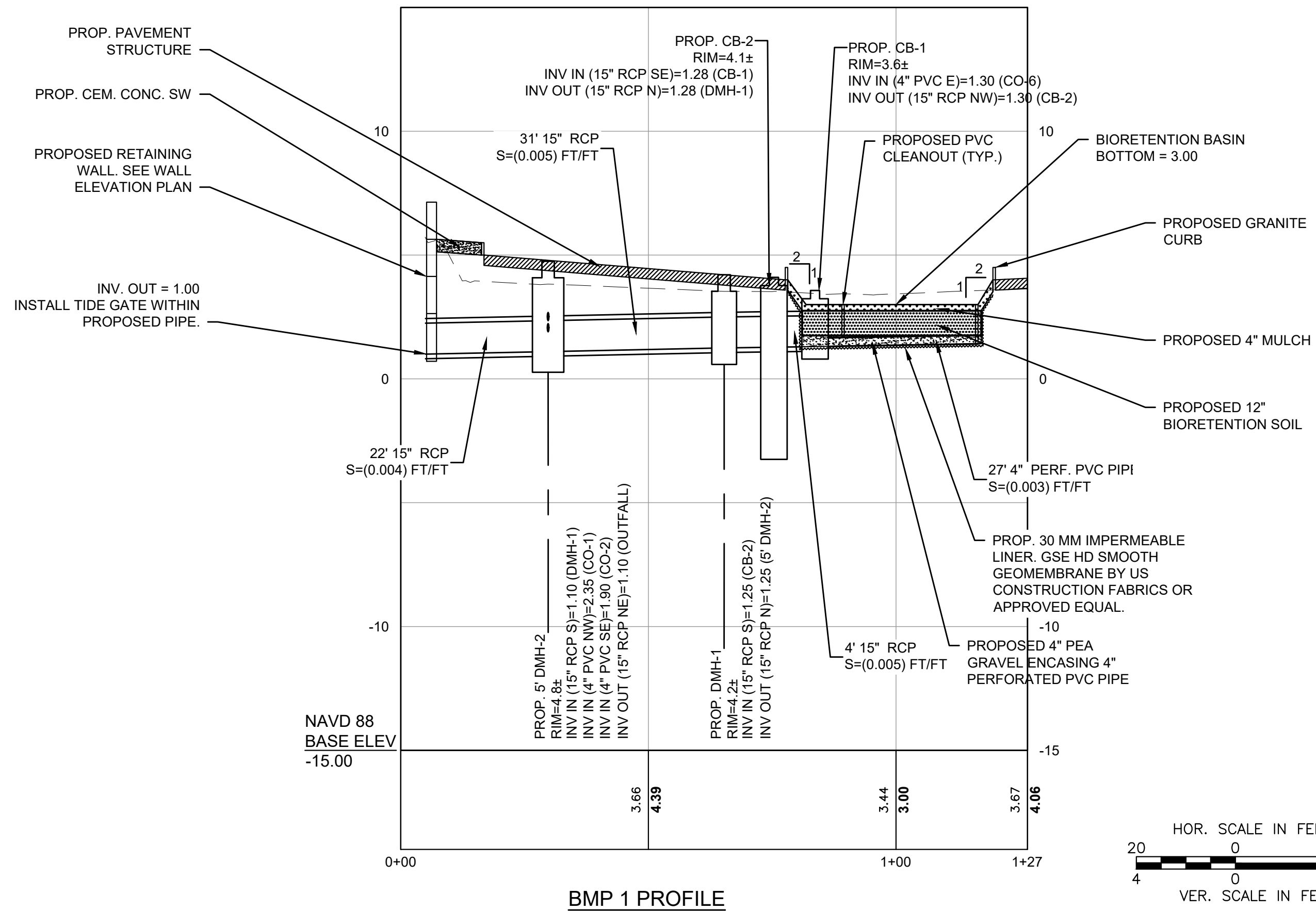
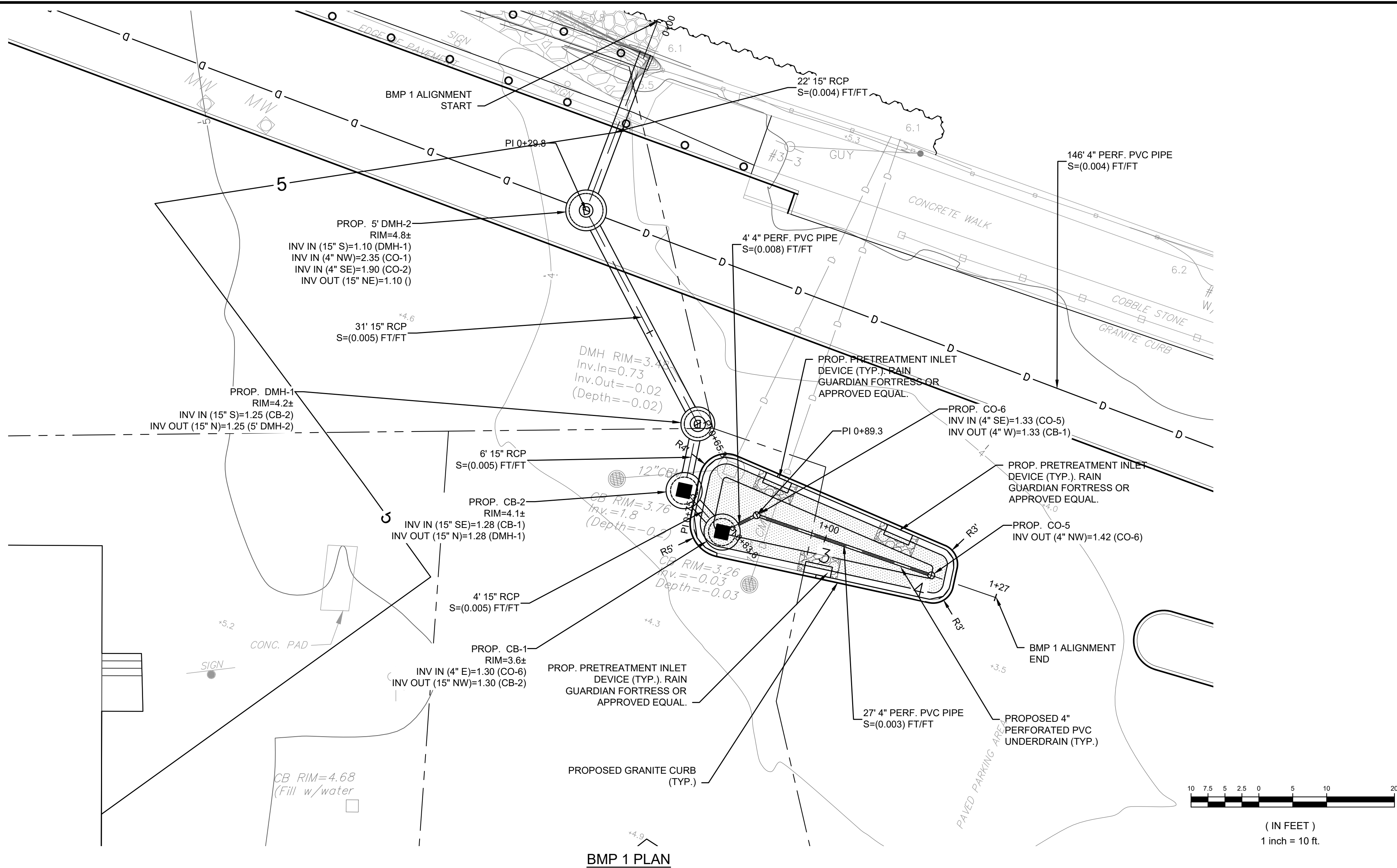
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NOTES:

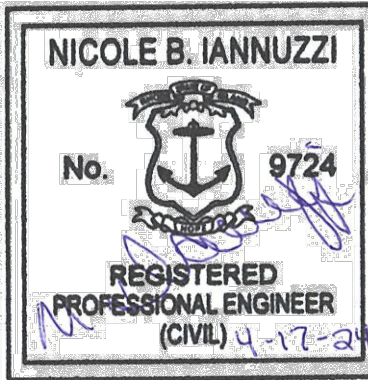
- BIORETENTION SOIL SHALL BE FREE OF STONES, ROCKS, COBBLES, BRUSH, STUMPS, ROOTS, AND OTHER MATERIALS LARGER THAN TWO INCHES. THE SOIL SHALL CONSIST OF USDA LOAMY SAND TO SANDY LOAM CLASSIFICATION AND MEET THE FOLLOWING GRADATION:
 - ASTM C-33 SAND = 68-72%
 - SILT = 6-10%
 - CLAY = 0-2%
 - ORGANIC MATTER (IN THE FORM OF WELL AERATED LEAF COMPOST) = 20%
- CONTRACTOR SHALL AVOID OVER COMPACTION OF SOILS WITH CONSTRUCTION VEHICLES WITHIN INFILTRATION AREAS.
- MULCH MUST BE FINELY SHREDDED, WELL AGED ORGANIC HARDWOOD
- PEA GRAVEL SHALL MEET THE GRADATION REQUIREMENTS SET FORTH IN SECTION M.01.09 TABLE 1 COLUMN III OF THE RHODE ISLAND DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS
- IMPERMEABLE LINER SHALL BE A NONWOVEN, HIGH-DENSITY POLYETHYLENE (HDPE) LINER SPECIFICALLY DESIGNED FOR USE IN SUBSURFACE LINING/CONTAINMENT.



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Wickford Waterfront Improvements

Wickford, RI

TITLE

BMP 1 DETAILS

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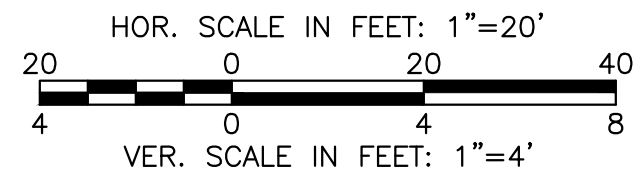
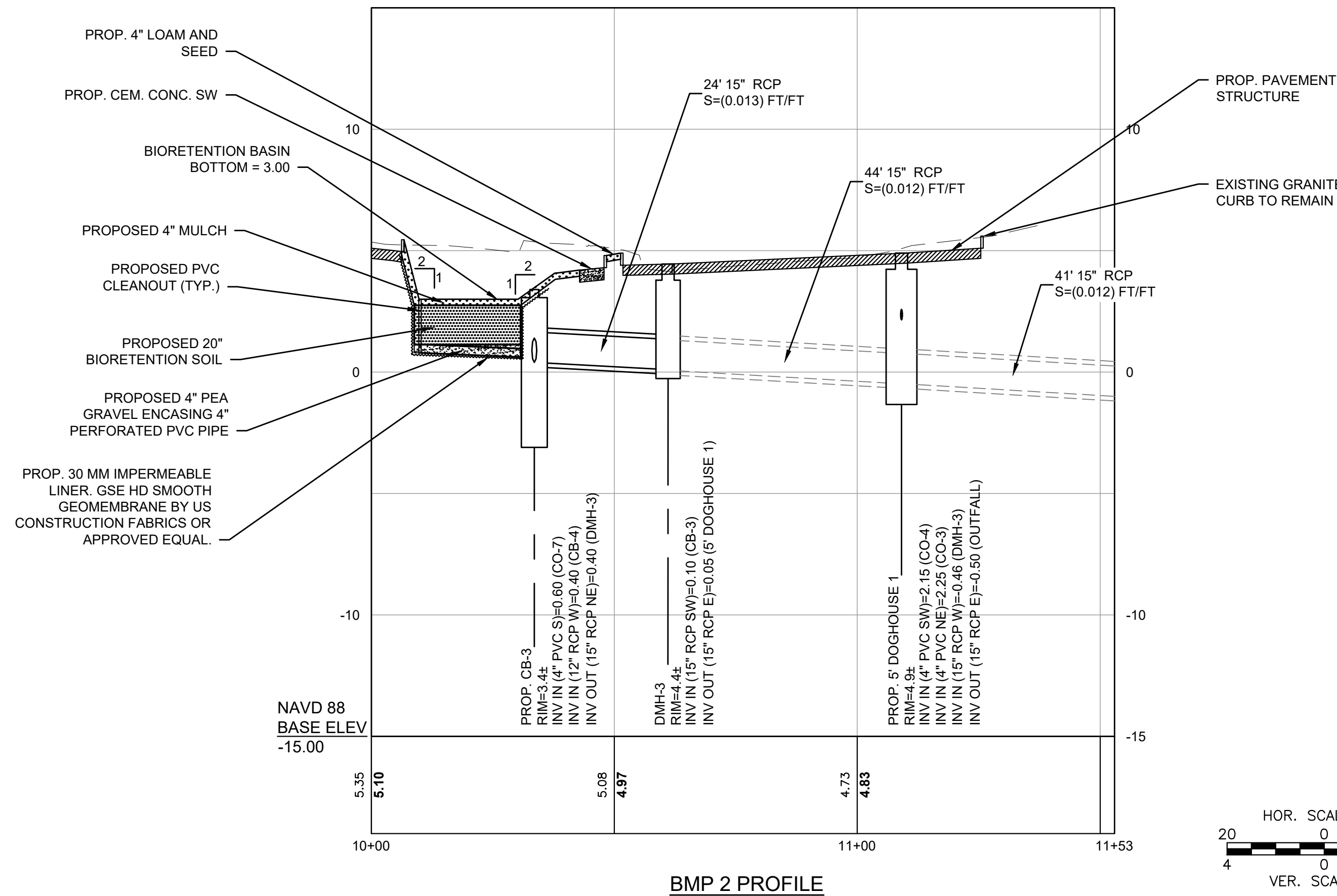
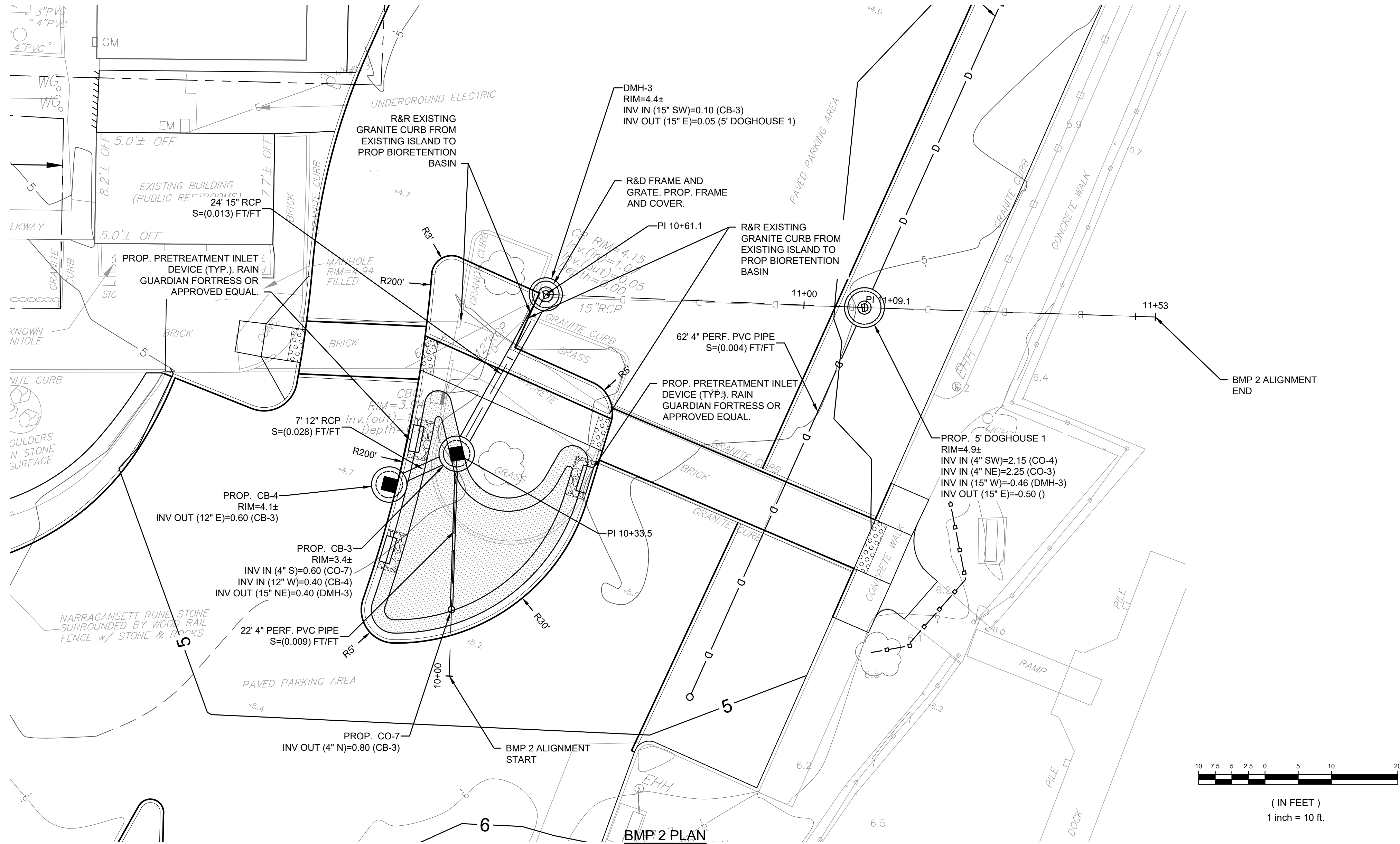
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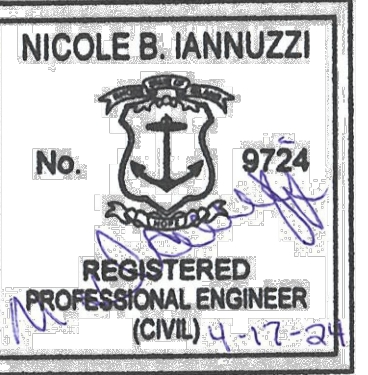
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Improvements

Wickford, RI

TITLE

BMP 2 DETAILS

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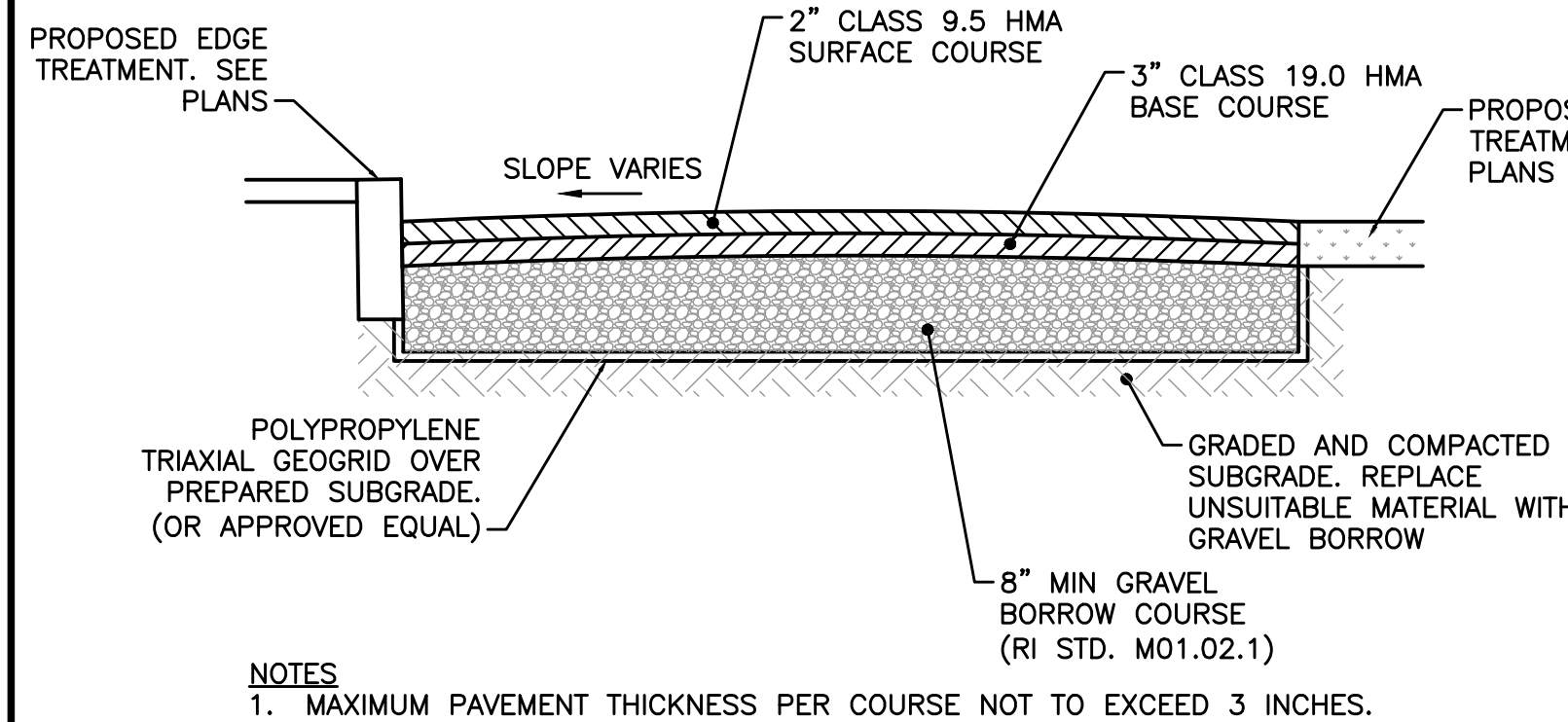
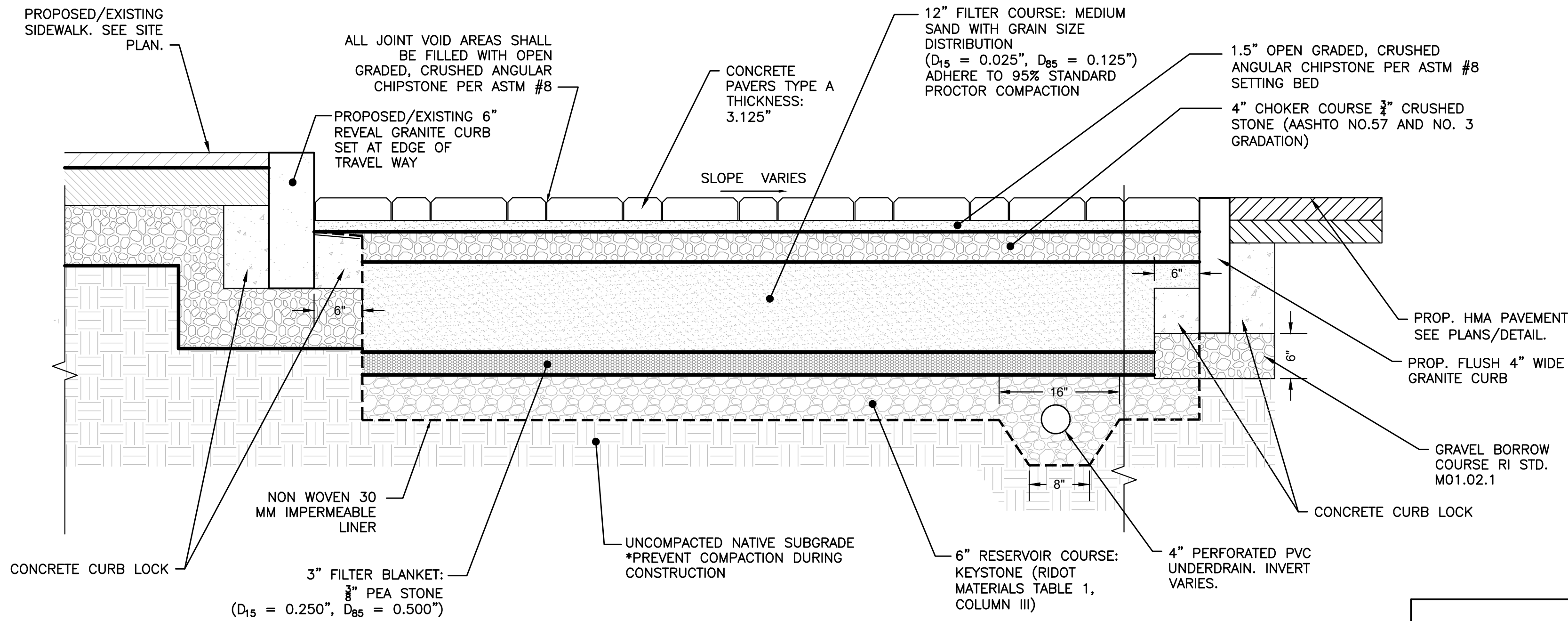
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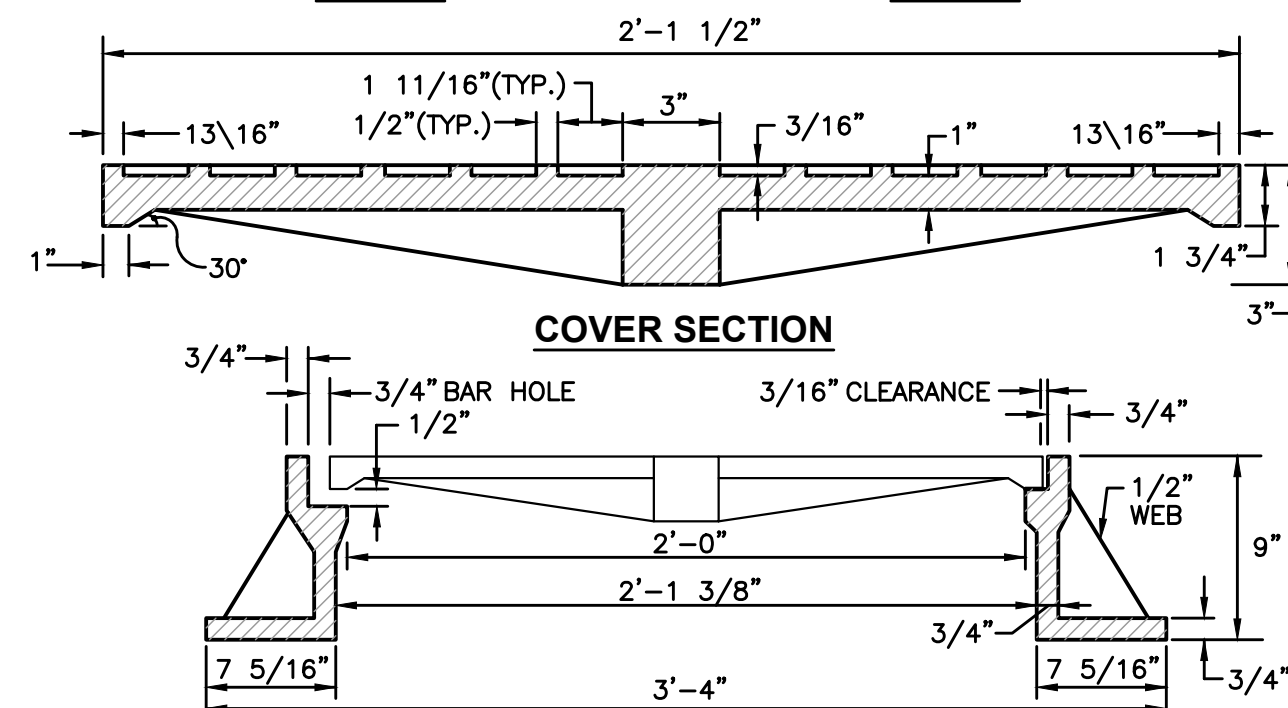
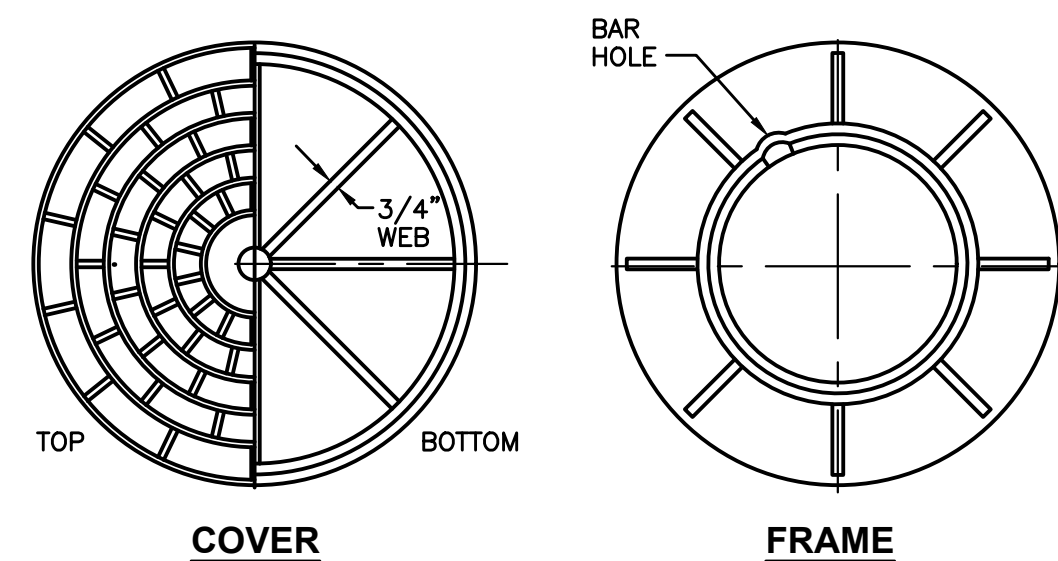
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4/16/2024 4:11 PM G:\PLANNING LANDSCAPE\70038 - WICKFORD WATERFRONT IMPROVEMENTS\AUTOCAD FILES\PLAN SET\7038-CONST DETAILS.DWG (BETA STD BW.CTB)



- NOTES:
1. MAXIMUM PAVEMENT THICKNESS PER COURSE NOT TO EXCEED 3 INCHES.

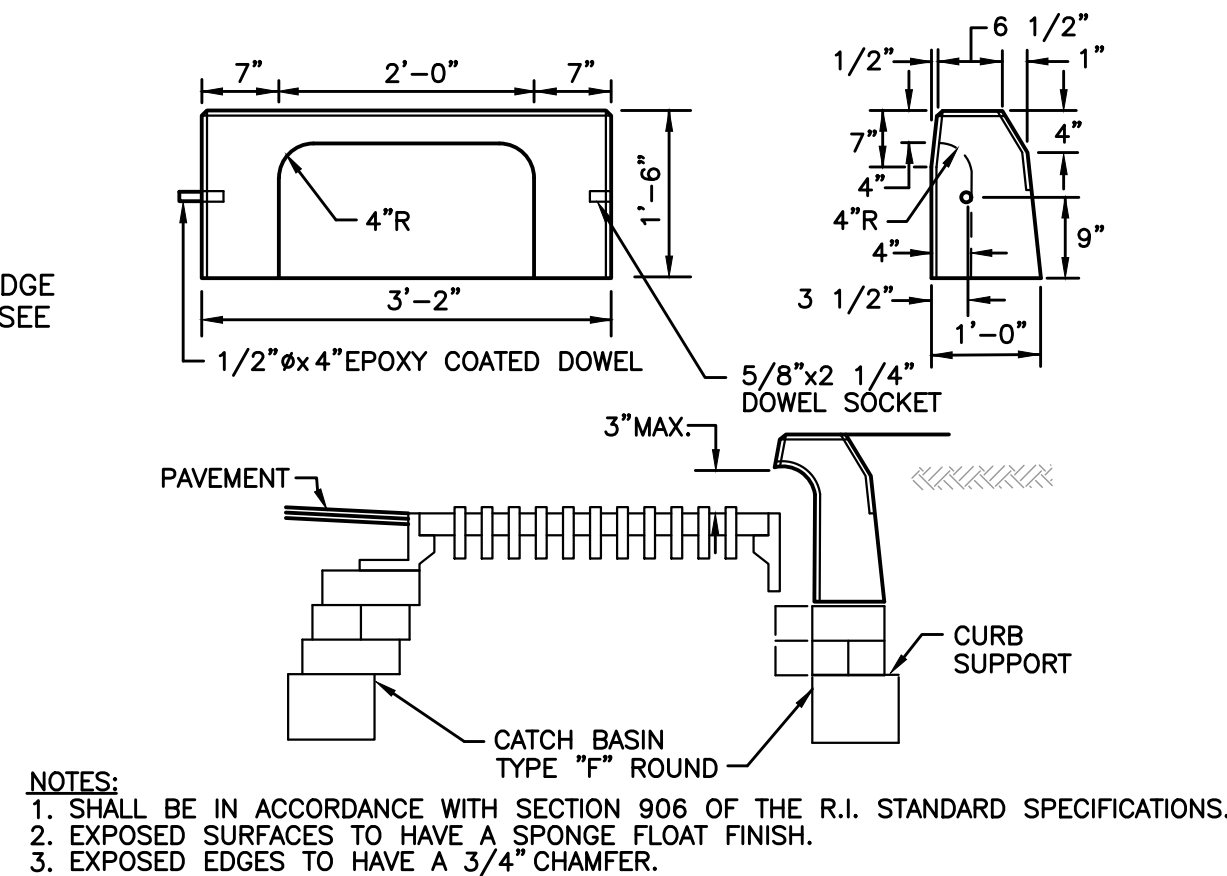


- NOTES:
1. FRAME AND GRATE SHALL CONFORM TO SECTION M.04 OF THE R.I. STANDARD SPECIFICATIONS.
 2. FRAME AND COVER SEATS MUST HAVE MACHINE FINISH.

**HEAVY-DUTY
ROUND FRAME AND COVER**
NOT TO SCALE

R.I.
STANDARD
6.2.1

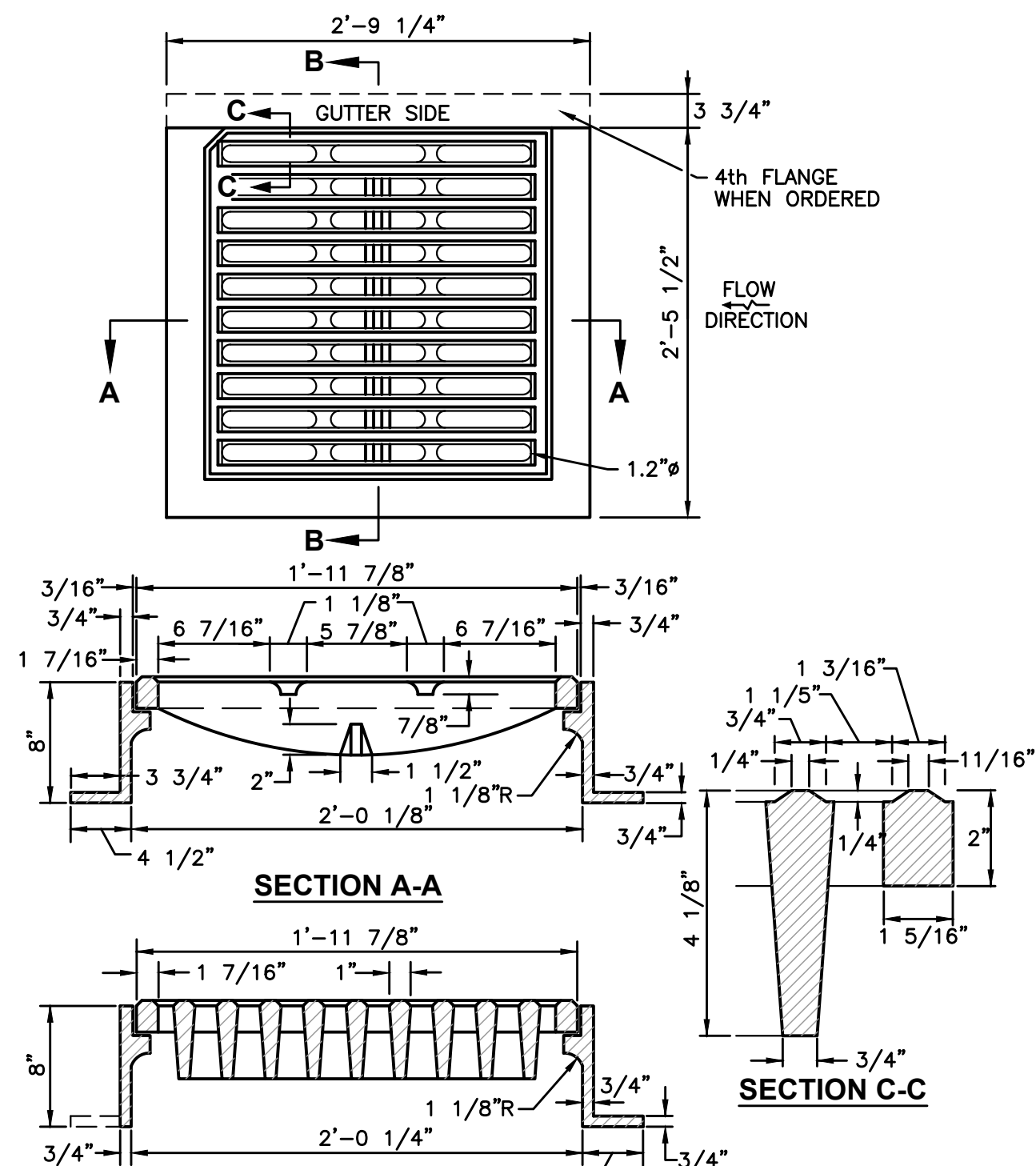
PAVING TYPE 'A' - PERMEABLE PAVERS
NOT TO SCALE



- NOTES:
1. SHALL BE IN ACCORDANCE WITH SECTION 906 OF THE R.I. STANDARD SPECIFICATIONS.
 2. EXPOSED SURFACES TO HAVE A SMOOTH FLOAT FINISH.
 3. EXPOSED EDGES TO HAVE A 3/4\"/>

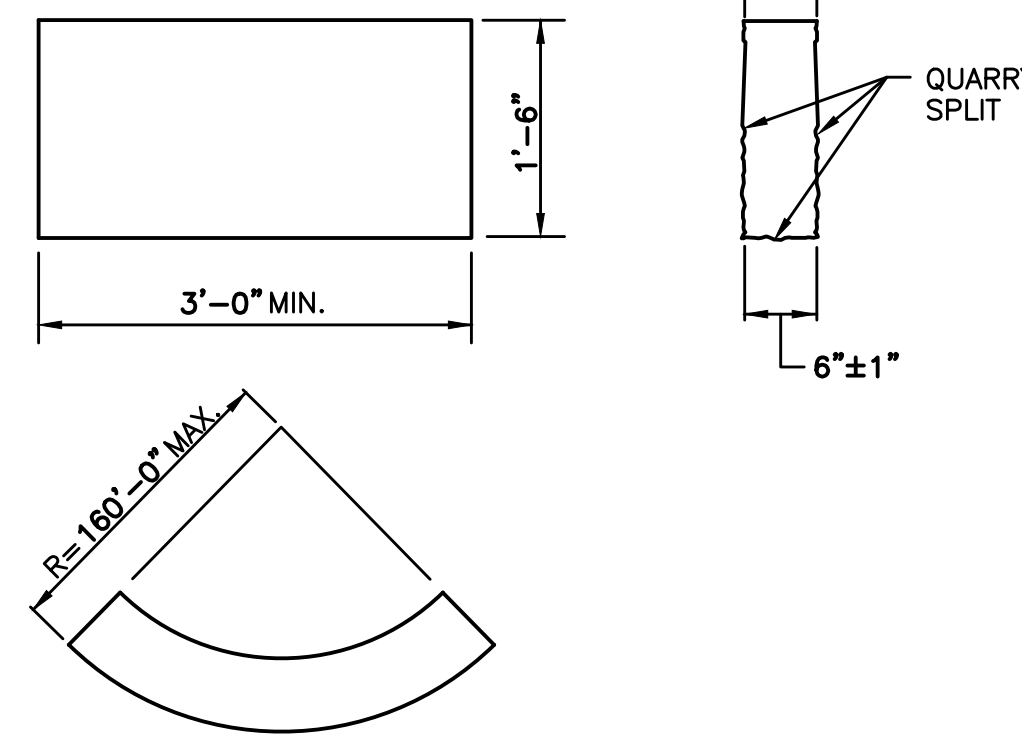
**PRECAST CONCRETE APRON STONE
(FOR ROUND CATCH BASIN)**
NOT TO SCALE

R.I.
STANDARD
7.1.8



**SQUARE FRAME AND GRATE
(BICYCLE SAFE)**
NOT TO SCALE

R.I.
STANDARD
6.3.2

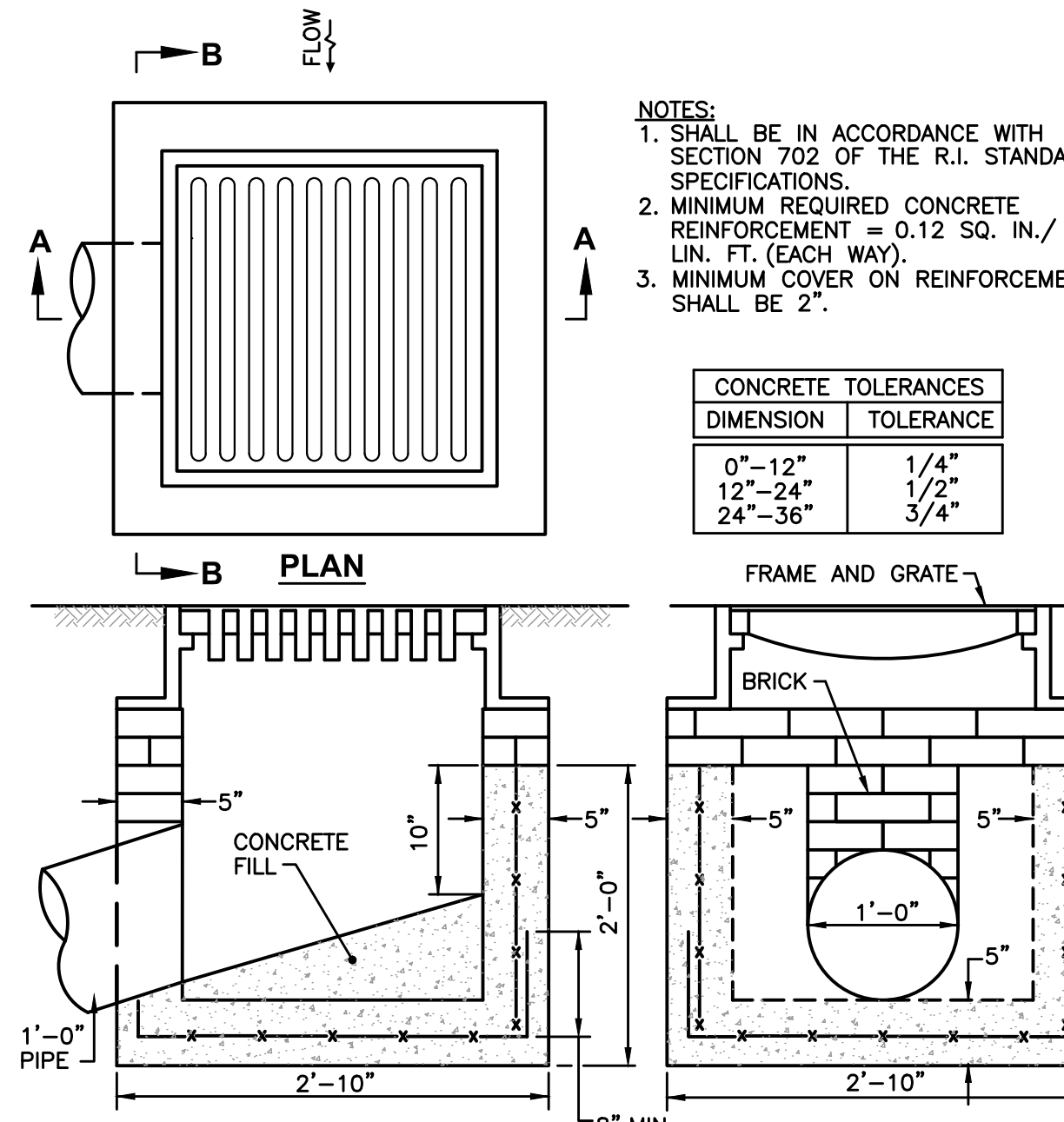


CIRCULAR CURB

- NOTES:
1. SHALL BE IN ACCORDANCE WITH SECTION 905 OF THE R.I. STANDARD SPECIFICATIONS.
 2. TOP SURFACE TO BE DRESSED BY SAW. REMAINDER TO BE QUARRY SPLIT.
 3. MINIMUM LENGTH OF STRAIGHT OR CIRCULAR PIECES TO BE 3'-0\"/>
 4. CIRCULAR CURB IS REQUIRED ON CURVES WITH RADII OF 160'-0\"/>

GRANITE CURB
NOT TO SCALE

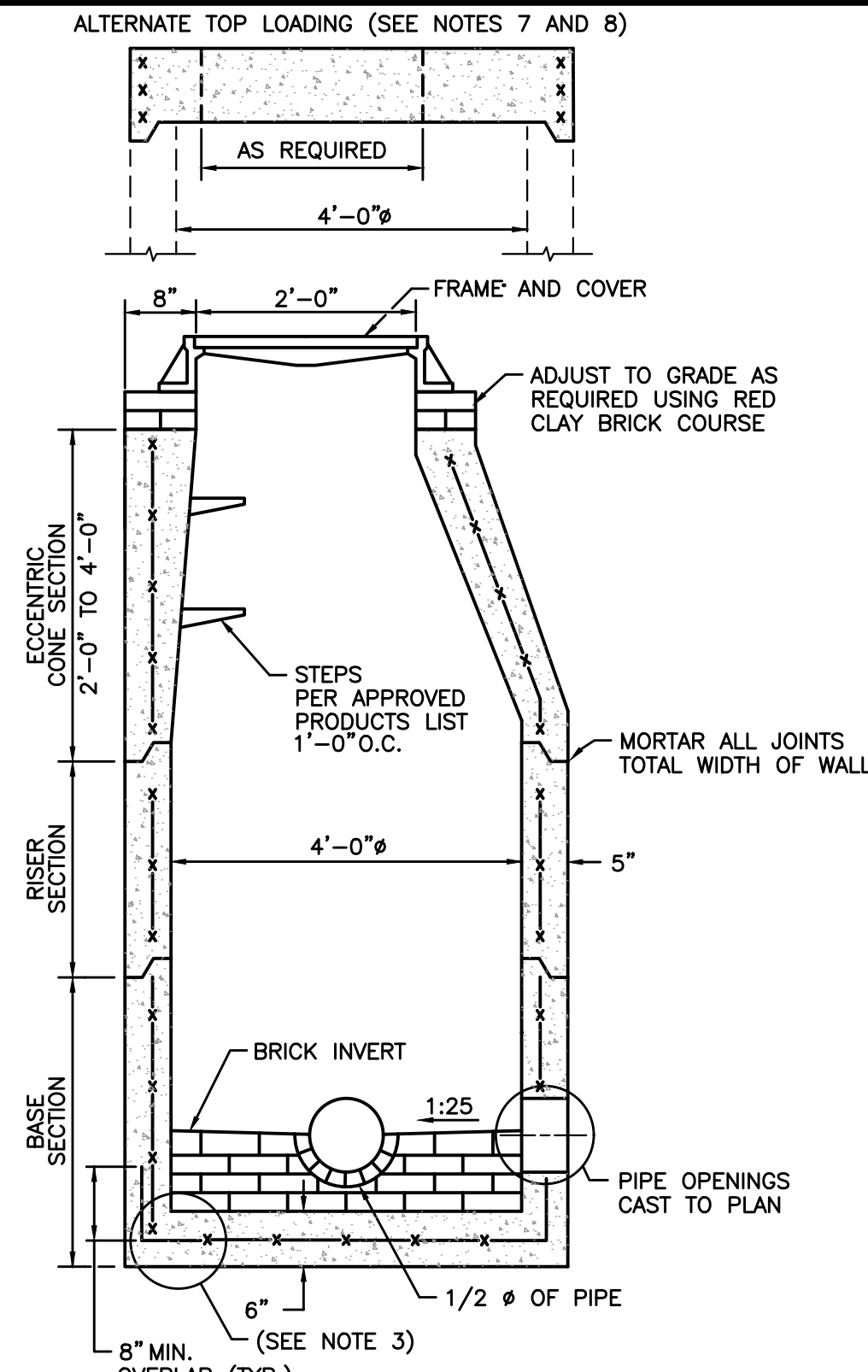
R.I.
STANDARD
7.3.0



SECTION A-A

PRECAST CONCRETE DROP INLET
NOT TO SCALE

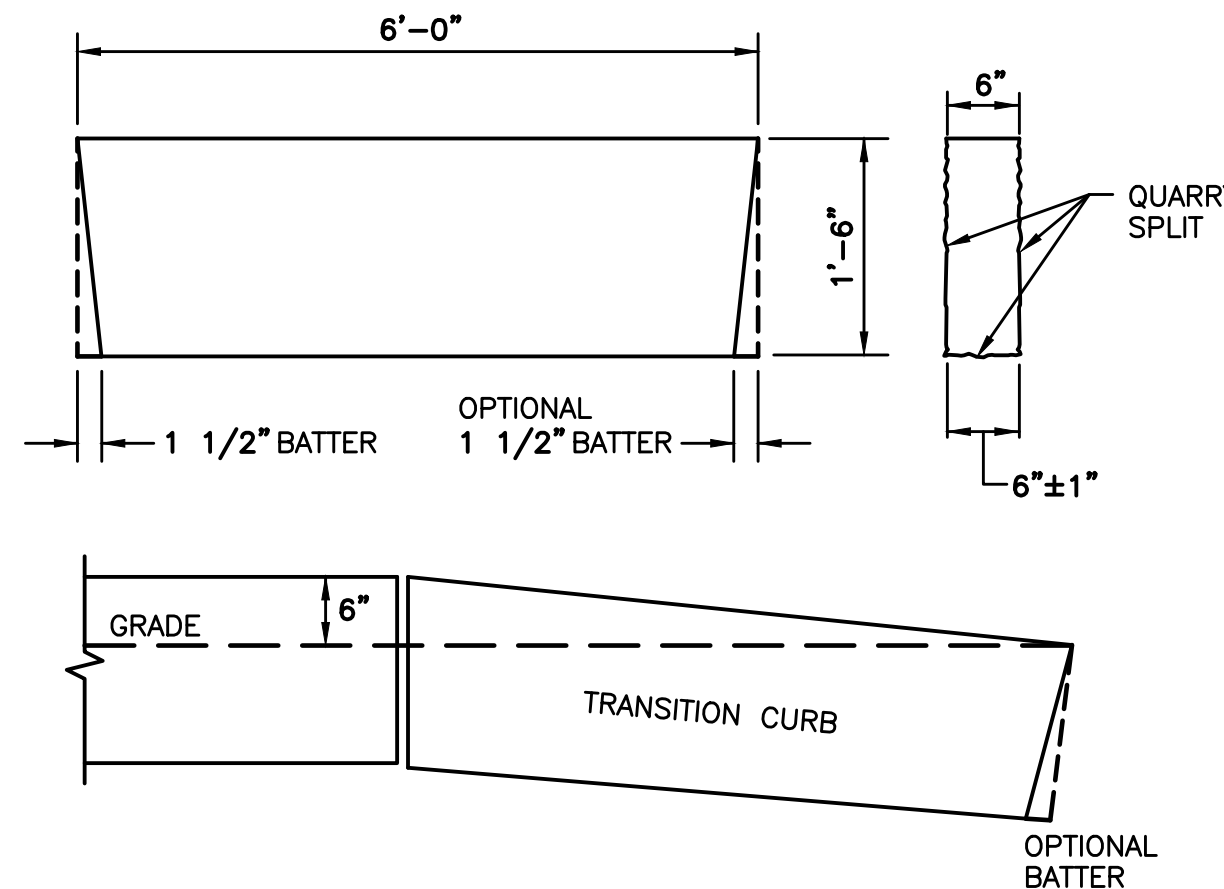
R.I.
STANDARD
4.5.0



- NOTES:
1. SHALL BE IN ACCORDANCE WITH SECTION 702 OF THE R.I. STANDARD SPECIFICATIONS.
 2. CIRCUMFERENTIAL STEEL REINFORCEMENT REQUIRED = 0.12 SQ. IN. / LIN. FT. MINIMUM.
 3. STEEL REINFORCEMENT FOR BASE SECTION BOTTOM SHALL BE A MINIMUM OF 0.12 SQ. IN./LIN. FT. (BOTH WAYS).
 4. ONE POUR MONOLITHIC BASE SECTION.
 5. ANY NECESSARY ADJUSTMENTS DURING CONSTRUCTION WILL BE DONE BY SAW-CUTTING AND/OR CORING ONLY. NO JACKHAMMERS, HAMMERS AND CHISELS OR PNEUMATIC TOOLS WILL BE ALLOWED.
 6. STEPS SHALL CONFORM TO STD. 5.3.0 AND SHALL BE INSTALLED AT THE CASTING PLANT.
 7. ALTERNATE TOP SLAB IS STEEL REINFORCED TO MEET OR EXCEED H-25 LOADING (SEE STD. 4.7.2).
 8. ALTERNATE TOP SLAB IS ONLY FOR USE WHEN REDUCING SECTION DOES NOT FIT BECAUSE OF STRUCTURE DEPTH.
 9. REFER TO STD. 5.2.0 FOR MAXIMUM PIPE SIZES.

PRECAST 4'-0\"/>

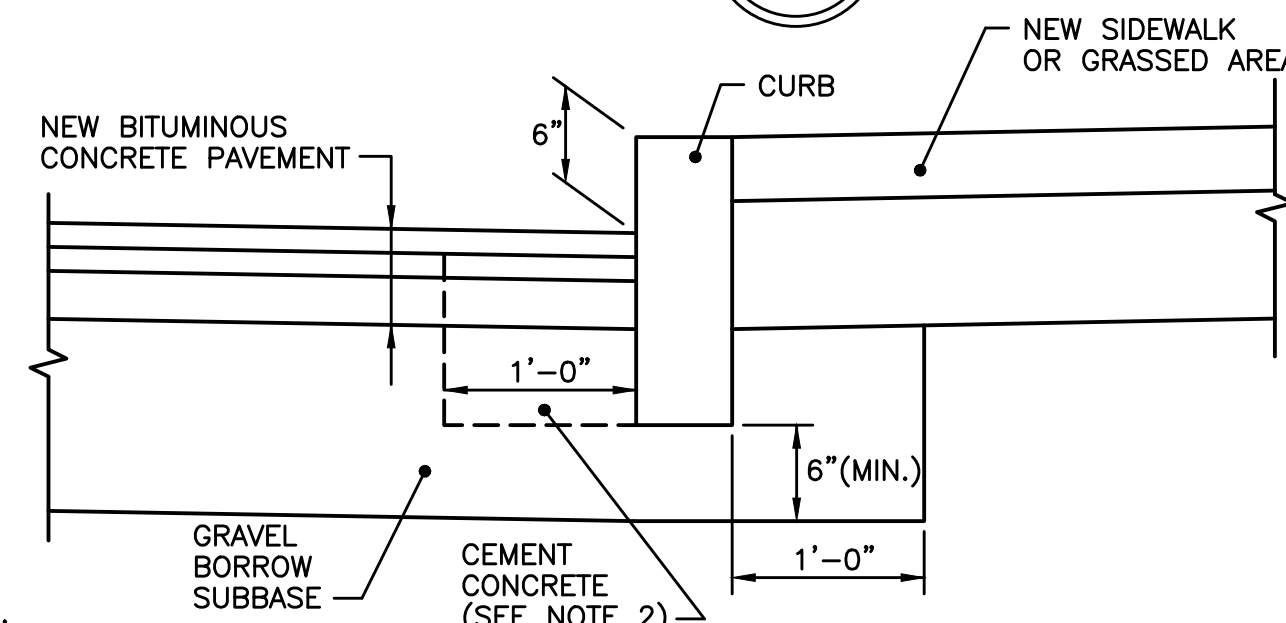
R.I.
STANDARD
4.2.0



- NOTES:
1. SHALL BE IN ACCORDANCE WITH SECTION 905 OF THE R.I. STANDARD SPECIFICATIONS.
 2. THE CONTRACTOR MAY CUT EXISTING CURB SECTIONS AS REQUIRED TO MEET THIS DETAIL AND THE R.I. STANDARD SPECIFICATIONS, WHERE OLD CURBING IS BEING REUSED.
 3. TOP SURFACE TO BE DRESSED BY SAW. REMAINDER TO BE QUARRY SPLIT.

6'-0\"/>

R.I.
STANDARD
7.3.0



- NOTES:
1. SHALL BE IN ACCORDANCE WITH SECTION 906 OF THE R.I. STANDARD SPECIFICATIONS.
 2. CEMENT CONCRETE SHALL BE USED ONLY WHEN THE CURB IS SET AFTER THE BASE AND/OR BINDER COURSES ARE IN PLACE, OTHERWISE THE CEMENT CONCRETE WILL BE ELIMINATED AND THE GRAVEL BROUGHT UP TO BOTTOM OF THE BASE COURSE.

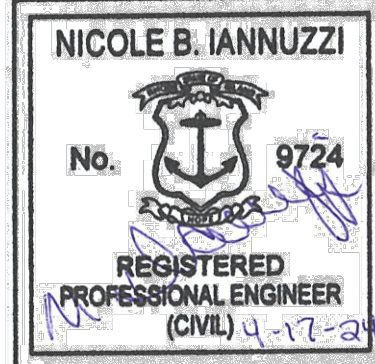
CURB SETTING DETAIL
NOT TO SCALE

R.I.
STANDARD
7.6.0

PREPARED BY



REGISTERED PROFESSIONAL



SUBCONSULTANT

PROJECT

**Wickford Waterfront
Improvements**

Wickford, RI

TITLE

**CONSTRUCTION
DETAILS 1**

NO.	REVISIONS	DATE
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DRAWN BY: CC

DESIGNED BY: TD

CHECKED BY: NBI

ISSUE DATE: 4/17/2024

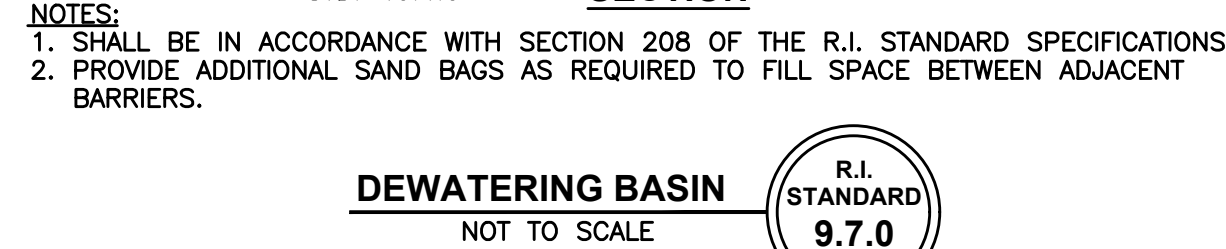
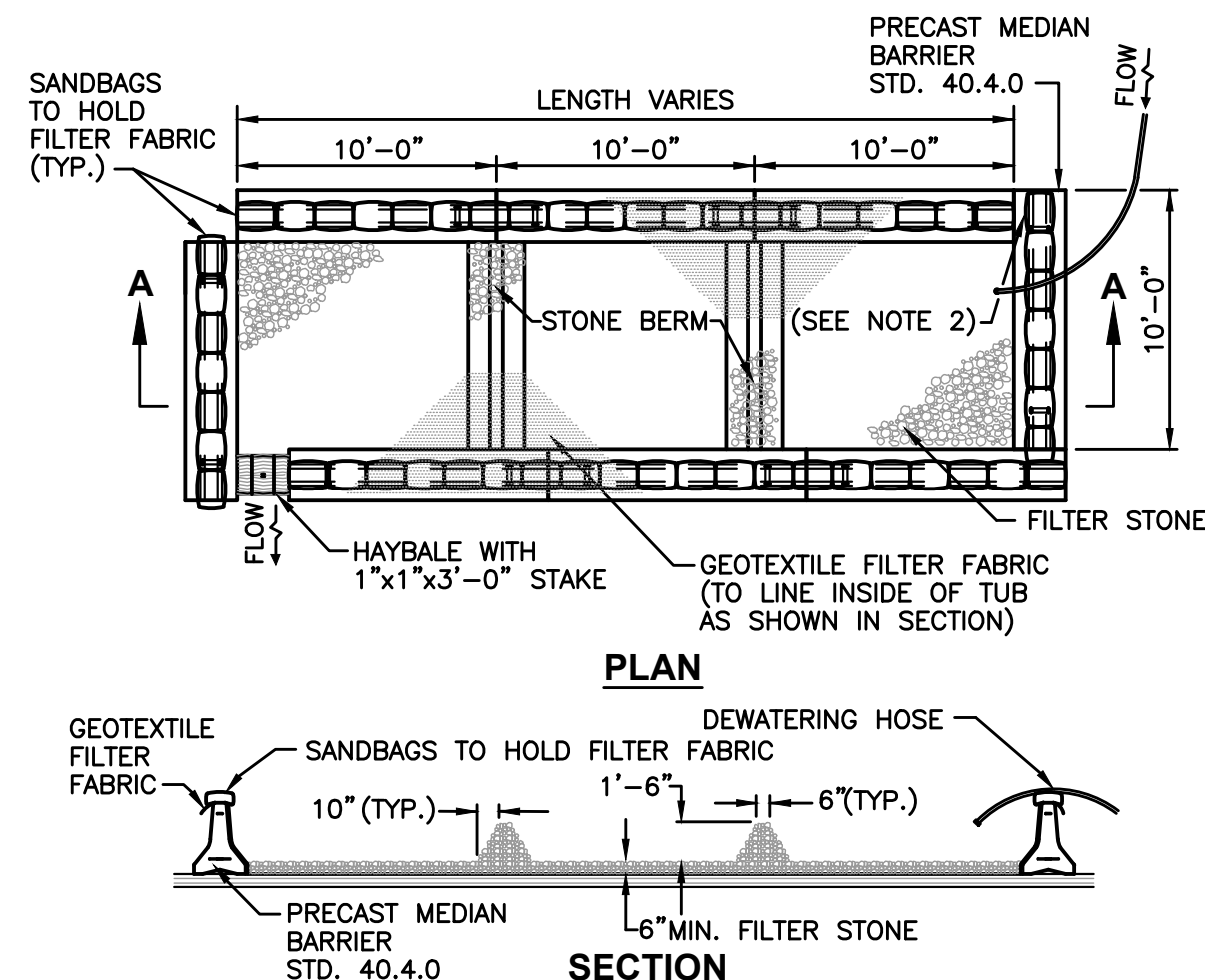
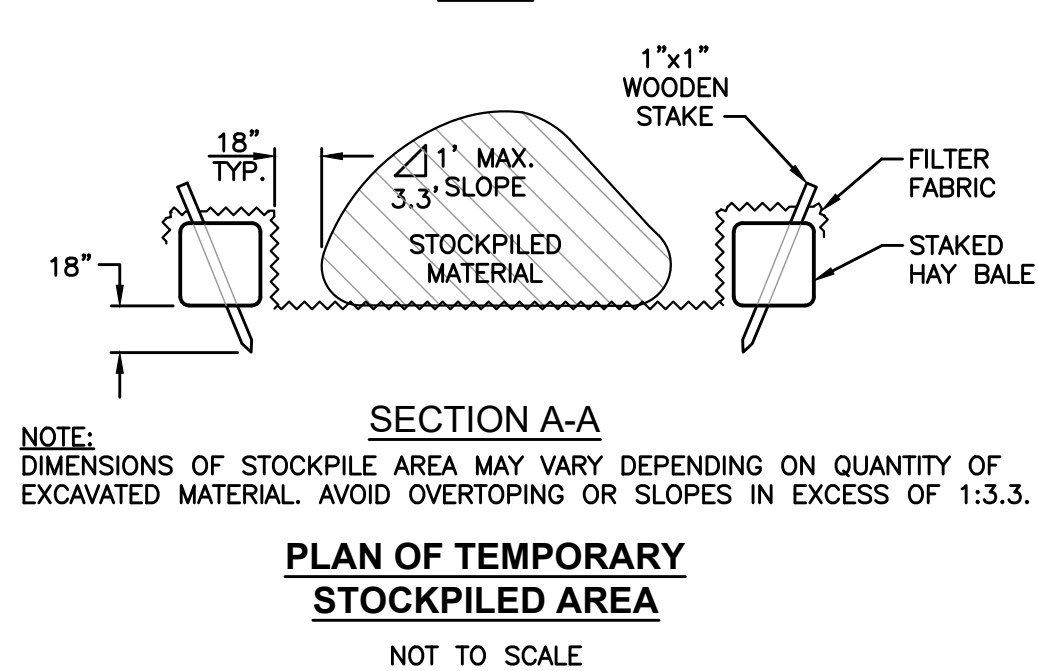
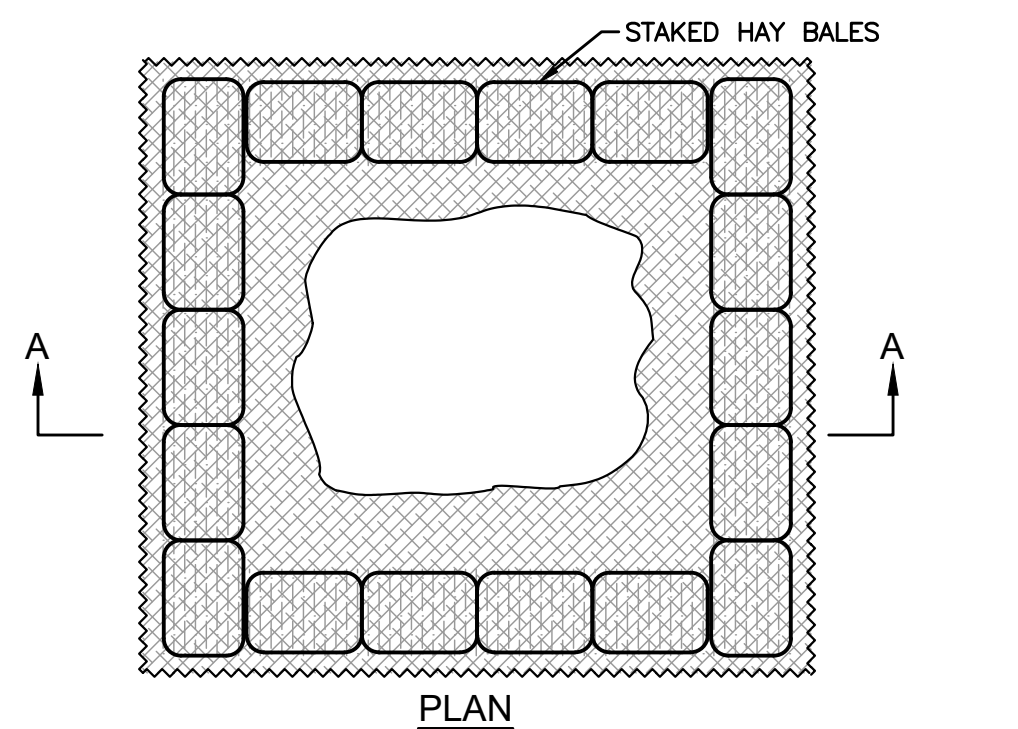
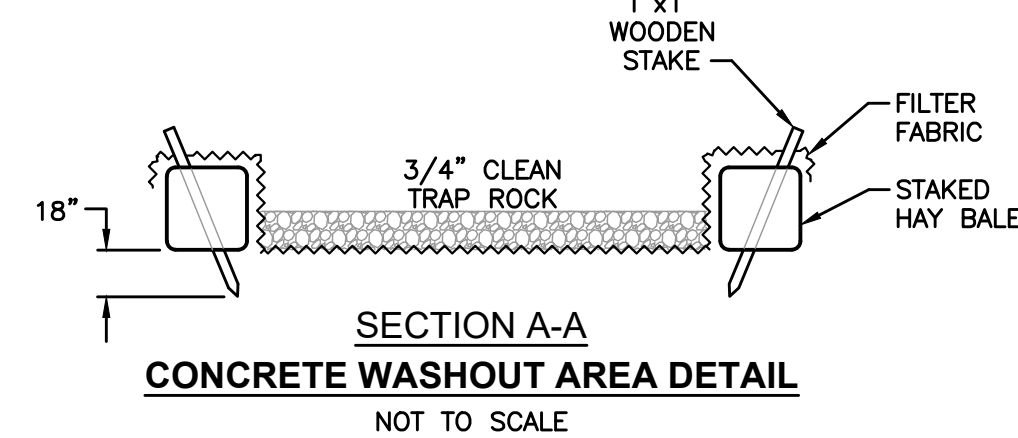
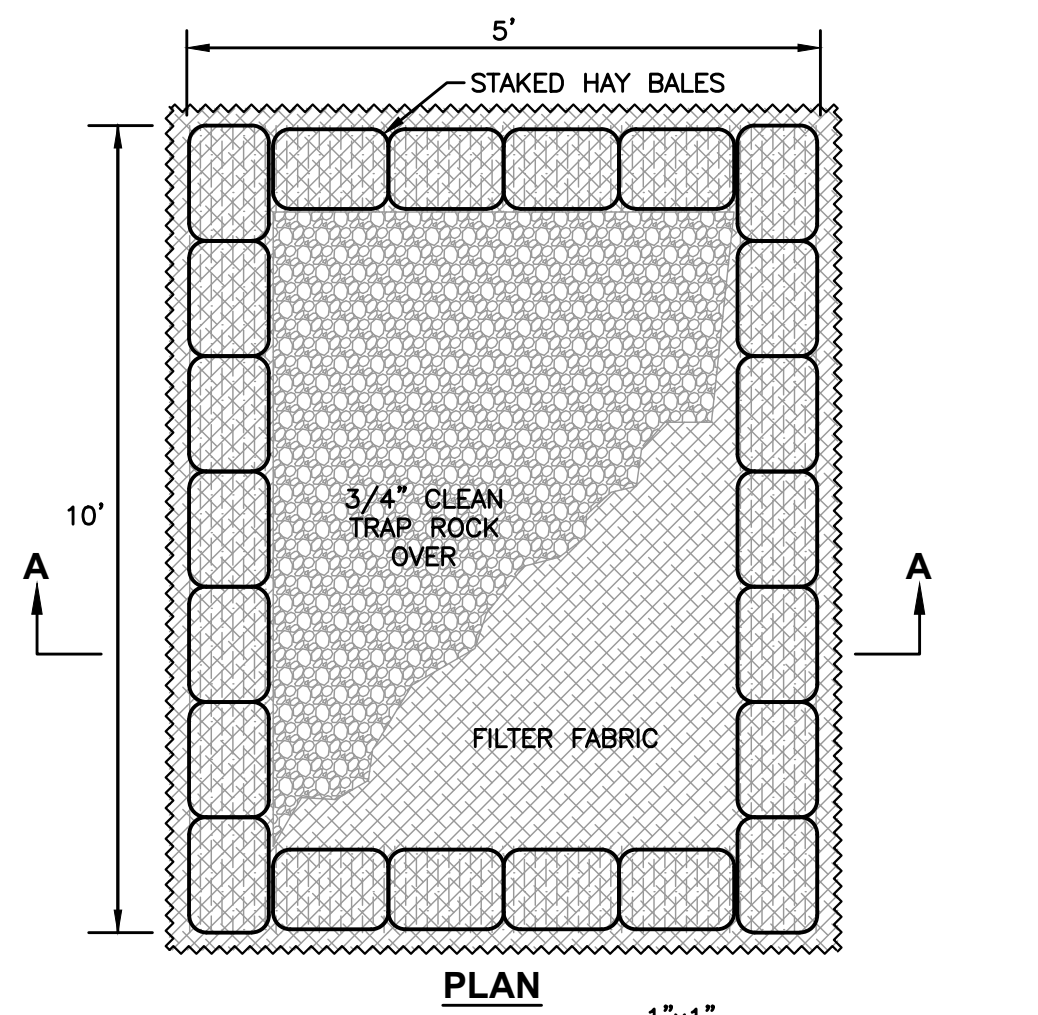
BETA JOB NO.: 7038

SCALE

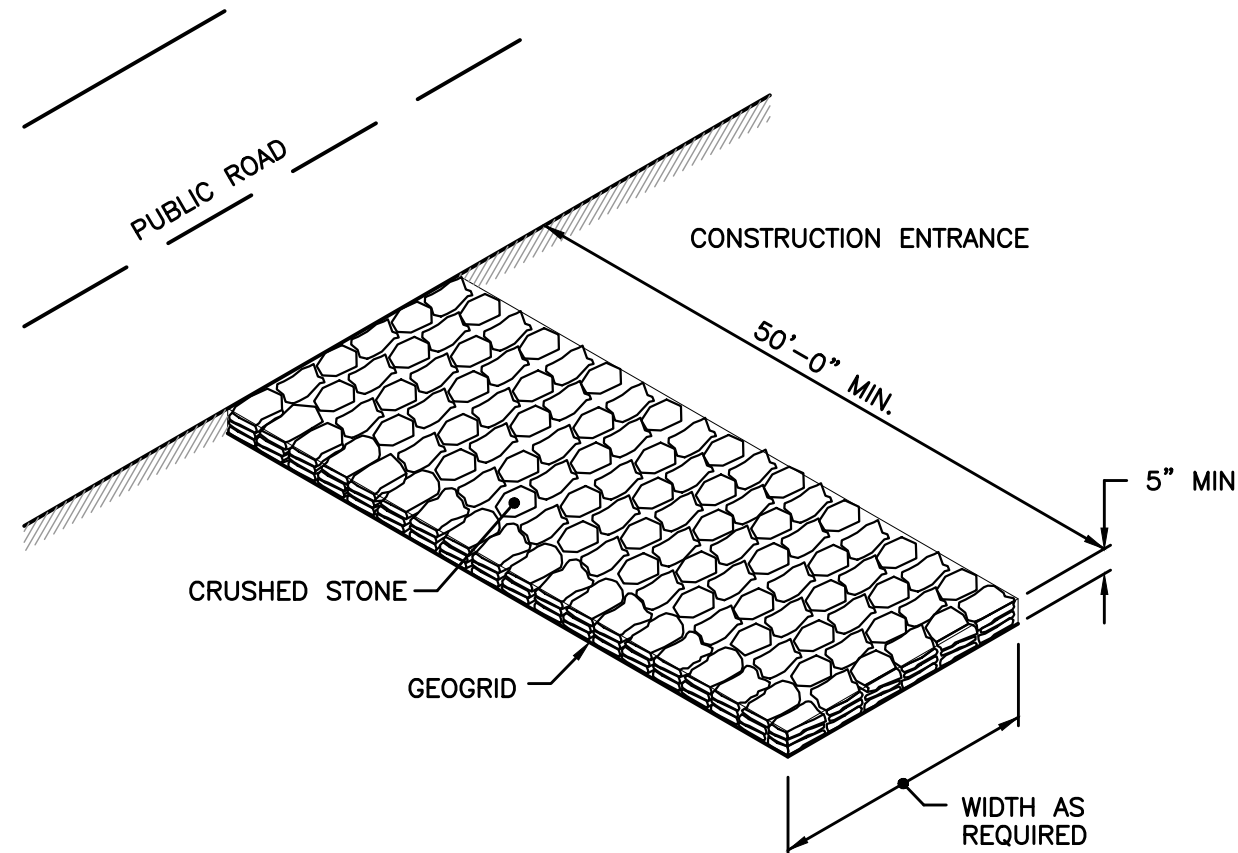
AS SHOWN

SHEET NO.

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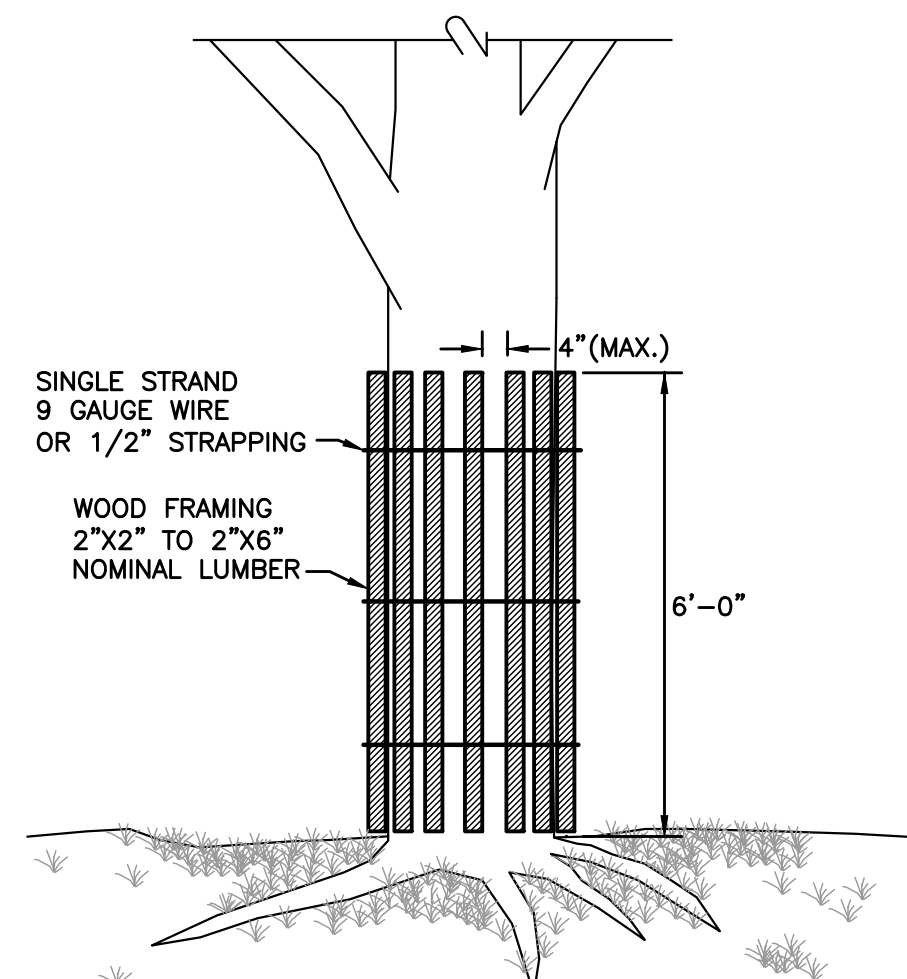


DEWATERING BASIN
NOT TO SCALE
R.I. STANDARD 9.7.0



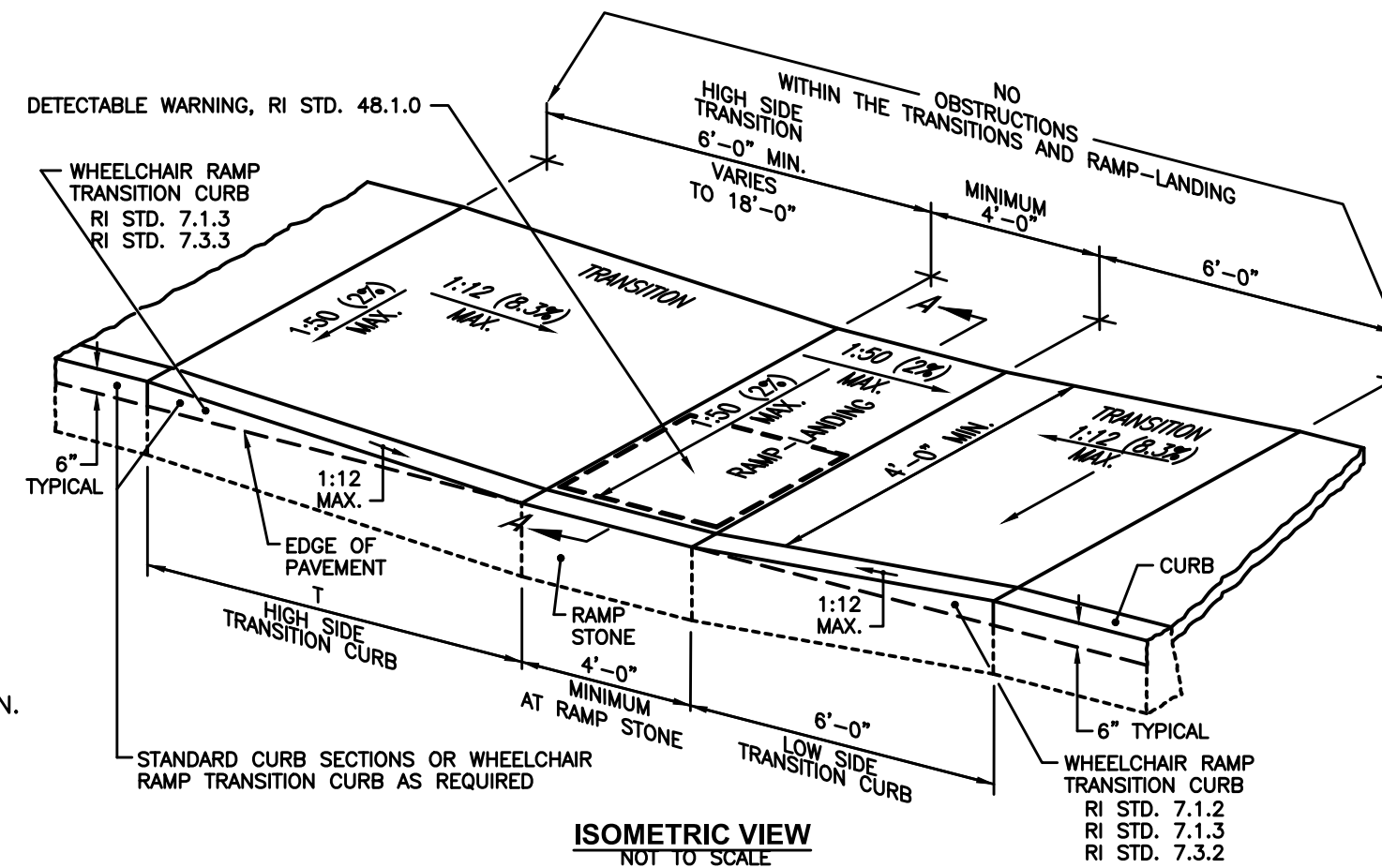
NOTE: SHALL BE IN ACCORDANCE WITH SECTION 211 OF THE R.I. STANDARD SPECIFICATIONS.

CONSTRUCTION ACCESS
NOT TO SCALE
R.I. STANDARD 9.9.0



NOTE: SHALL BE IN ACCORDANCE WITH SECTION L.11 OF THE STANDARD SPECIFICATIONS.

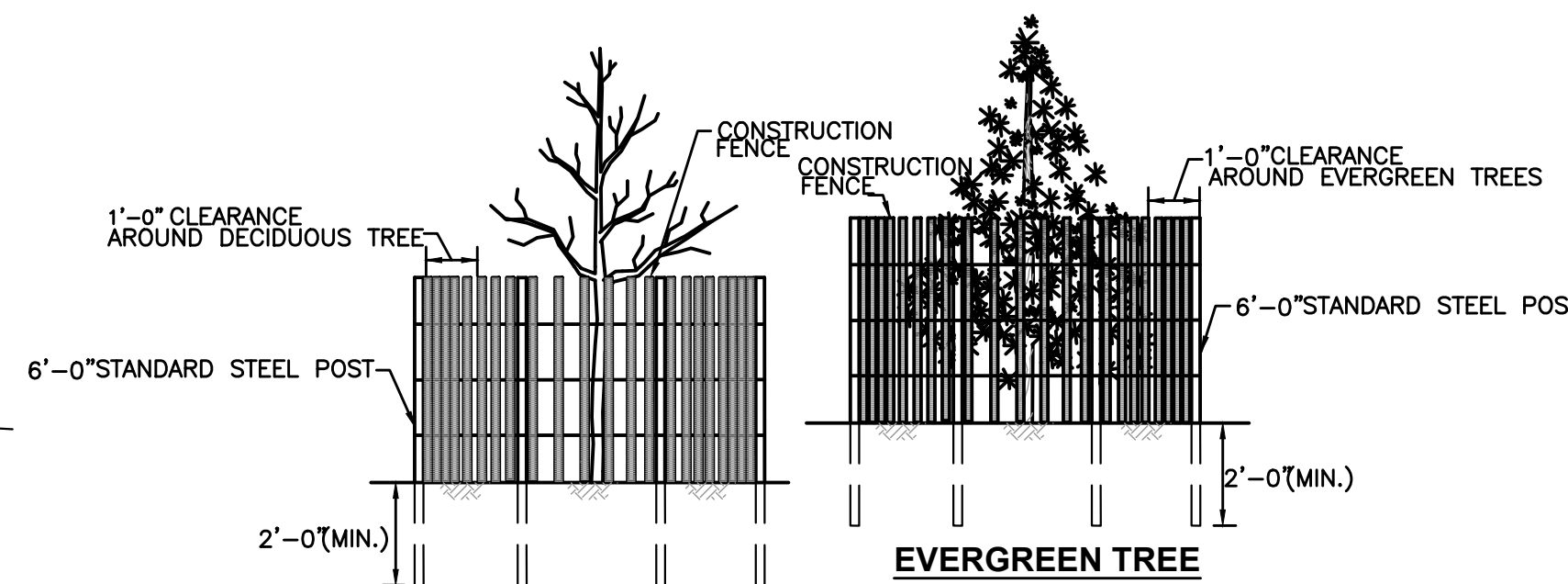
TREE PROTECTION DEVICE
NOT TO SCALE
R.I. STANDARD 51.1.0



ROADWAY PROFILE GRADE	T (FT.)
0.00	6.0
0.01	7.0
0.02	8.0
0.03	9.5
0.04	11.5
0.05	15.0
> 0.05	18.0

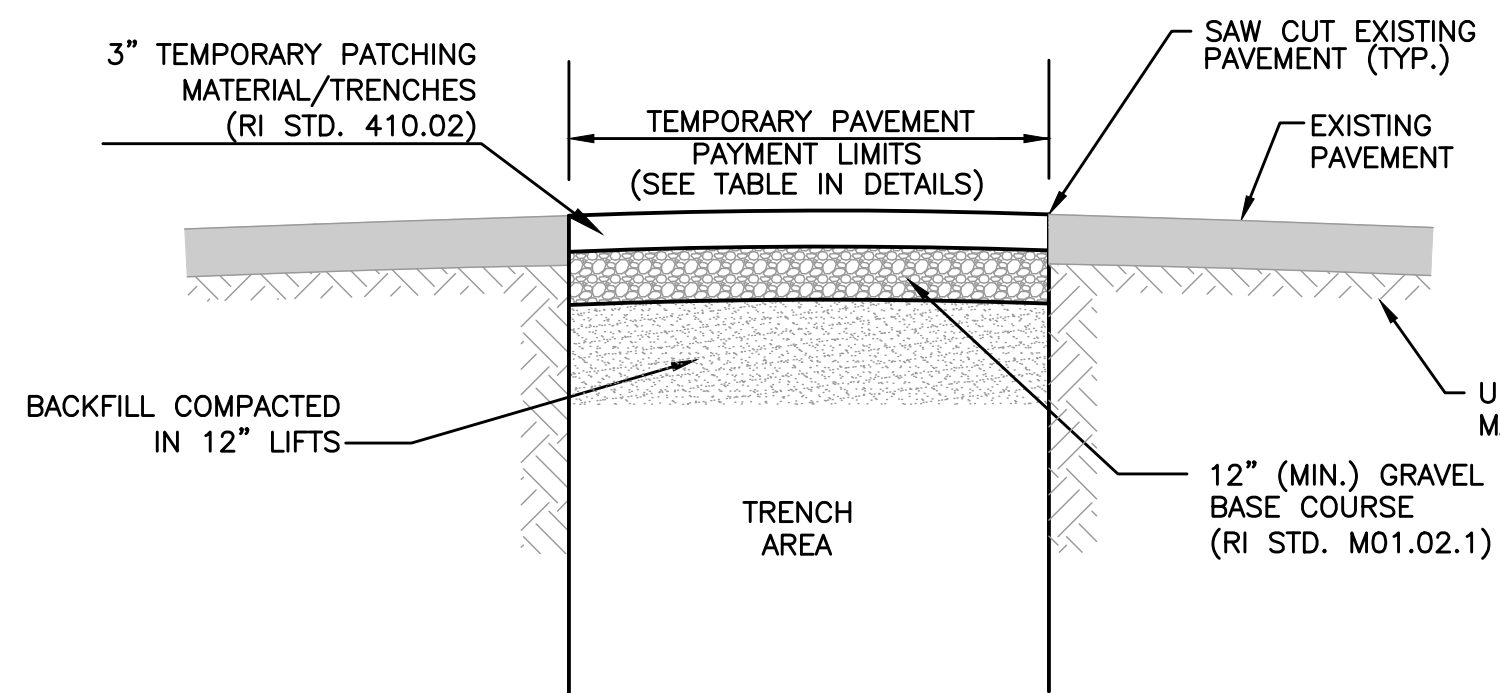
- NOTES:
1. THIS DETAIL MAY BE USED WHEN A PHYSICAL BARRIER IS PRESENT AND THERE IS INSUFFICIENT ROOM TO PROPERLY CONSTRUCT AN ADA ACCESSIBLE RAMP AND LANDING; A TECHNICAL FEASIBILITY FINDING IS REQUIRED.
 2. SHALL BE IN ACCORDANCE WITH SECTION 905 OF THE R.I. STANDARD SPECIFICATIONS.
 3. THE RAMP-LANDING AND TRANSITIONS SHALL BE FREE OF OBSTRUCTIONS.
 4. LOCATION OF THE RAMP-LANDING IS AS SHOWN ON THE CONTRACT DRAWINGS.
 5. AN UNOBSTRUCTED PEDESTRIAN ACCESS ROUTE (PATH OF TRAVEL) WITH A MINIMUM WIDTH OF 4'-0\"/>

RAMP-LANDING FOR NARROW SIDEWALK
NOT TO SCALE
R.I. STANDARD 43.3.2



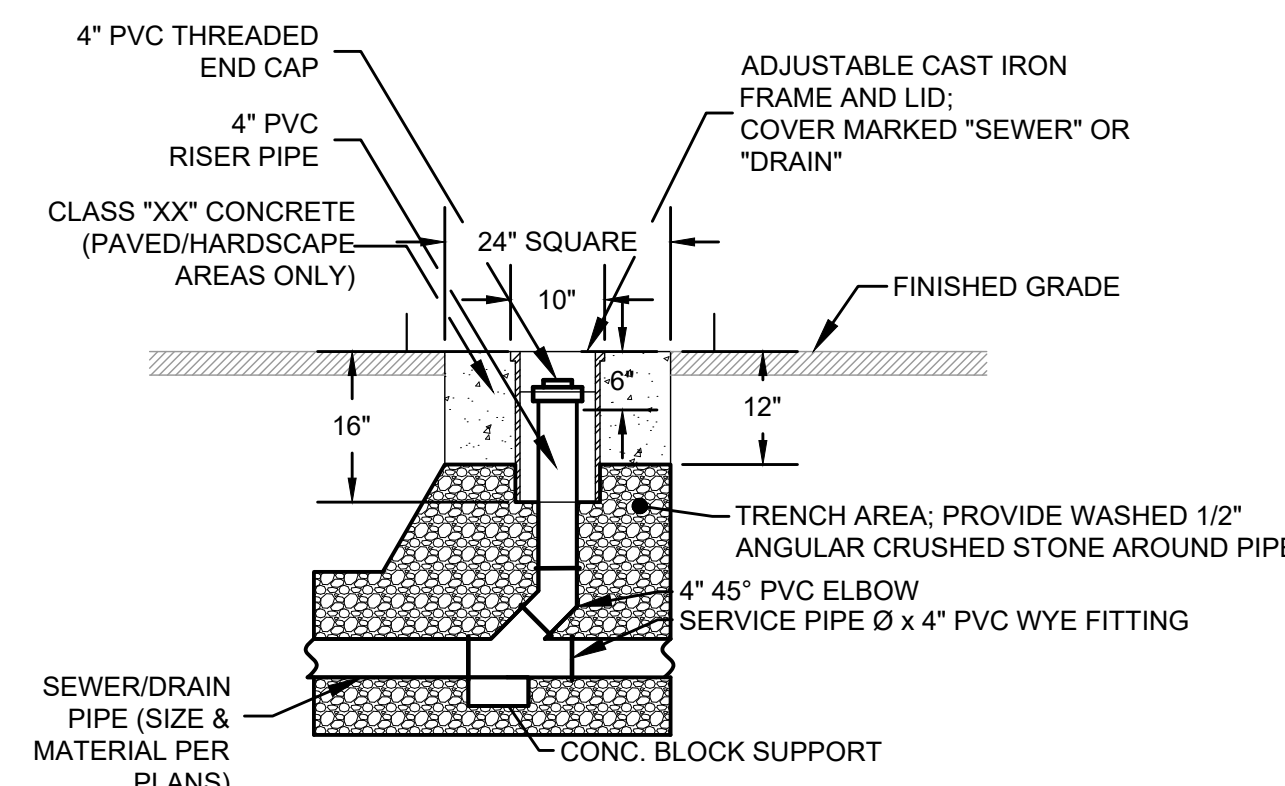
NOTE: SHALL BE IN ACCORDANCE WITH SECTION L.11 OF THE R.I. STANDARD SPECIFICATIONS.

DRIP LINE TREE PROTECTION DEVICE FOR EXISTING TREES
NOT TO SCALE
R.I. STANDARD 51.1.1

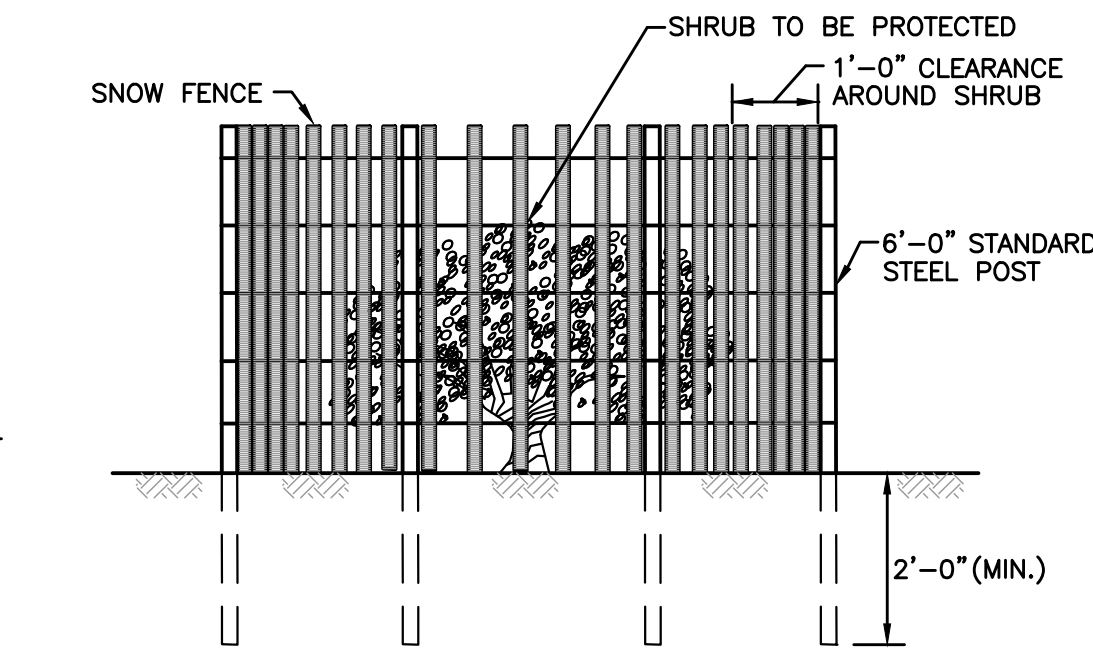


NOTE: CONTRACTOR TO VARY PAVEMENT THICKNESS TO MAINTAIN A MINIMUM CROSS SECTIONAL SLOPE EQUALING 0.02 FT/FT OR %.

BITUMINOUS CONCRETE TEMPORARY PAVEMENT
NOT TO SCALE

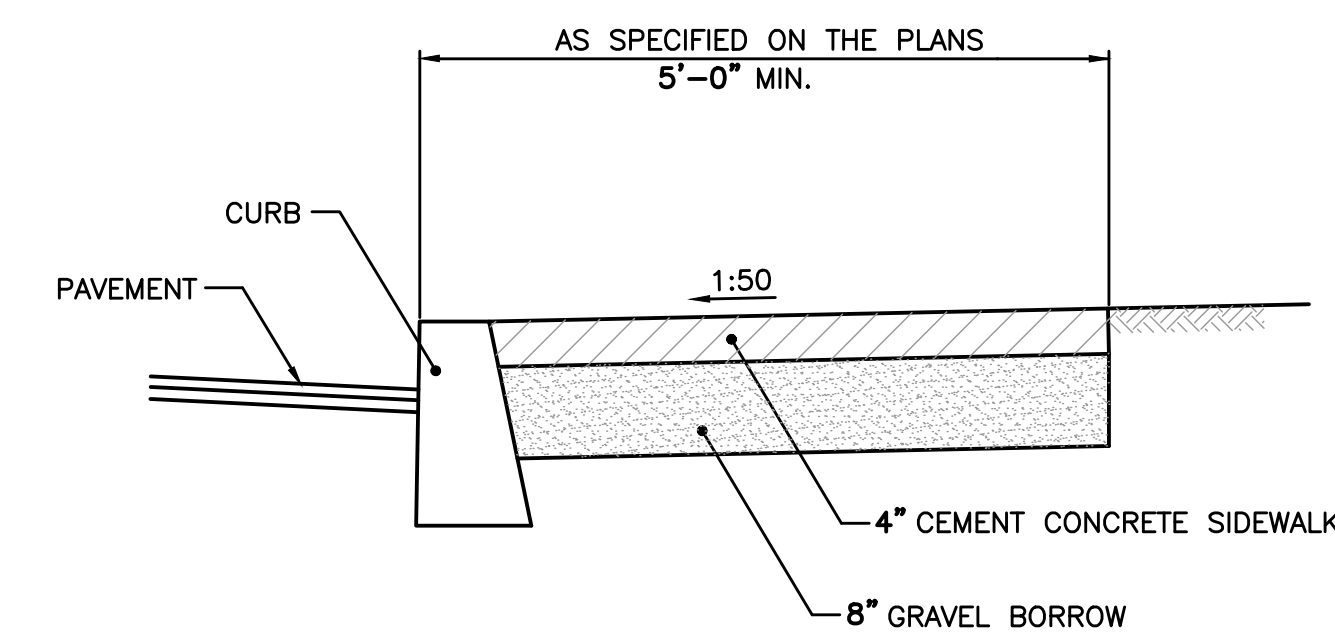


TYPICAL SEWER/ROOF DRAIN CLEANOUT DETAIL
NOT TO SCALE



NOTE: SHALL BE IN ACCORDANCE WITH SECTION L.11 OF THE R.I. STANDARD SPECIFICATIONS.

SHRUB PROTECTION DEVICE
NOT TO SCALE
R.I. STANDARD 51.2.0



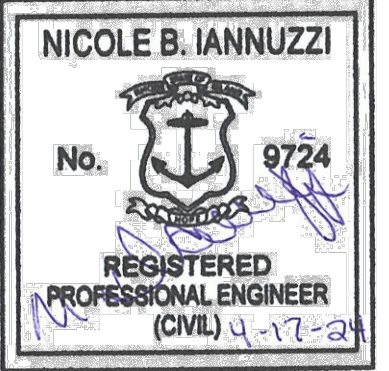
- NOTES
1. SHALL BE IN ACCORDANCE WITH SECTION 904 OF THE R.I. STANDARD SPECIFICATIONS.
 2. FOR CURB SETTING DETAIL REFERENCE STD. 7.6.0.

CEMENT CONCRETE SIDEWALK
NOT TO SCALE
R.I. STANDARD 43.1.0

PREPARED BY



REGISTERED PROFESSIONAL



SUBCONSULTANT

PROJECT

Wickford Waterfront Improvements

Wickford, RI

TITLE

CONSTRUCTION DETAILS 2

NO.	REVISIONS	DATE
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DRAWN BY: CC

DESIGNED BY: TD

CHECKED BY: NBI

ISSUE DATE: 4/17/2024

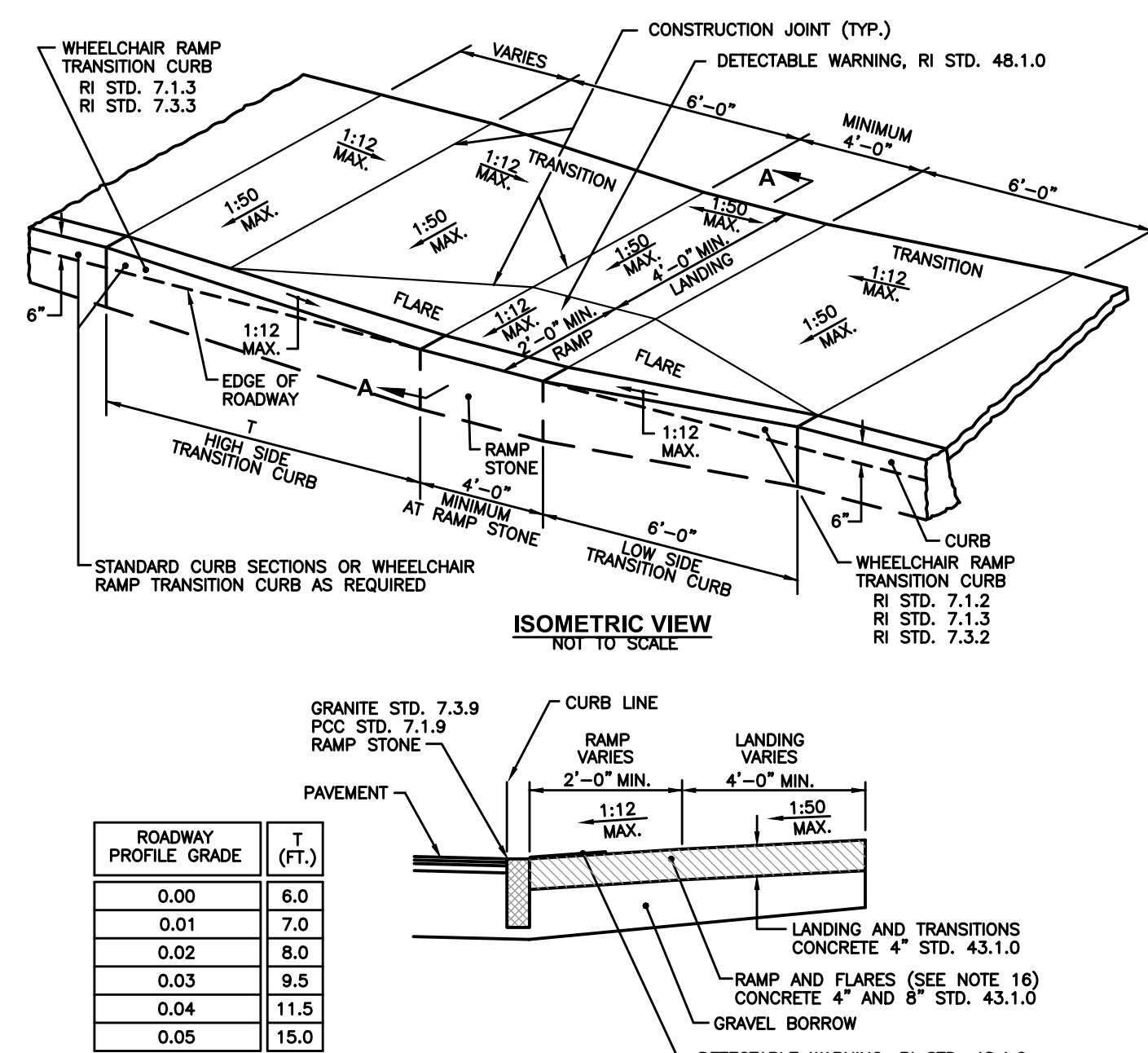
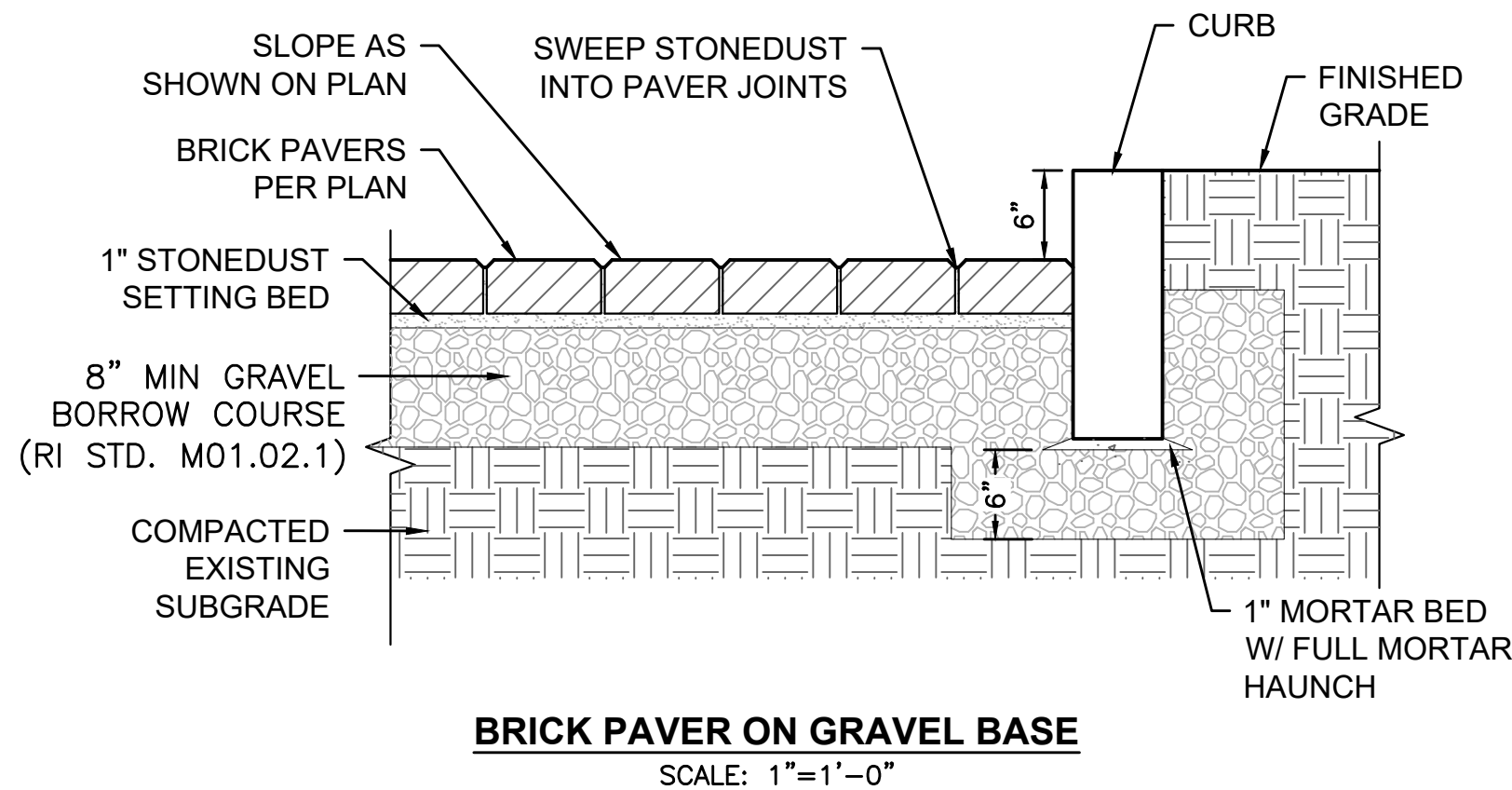
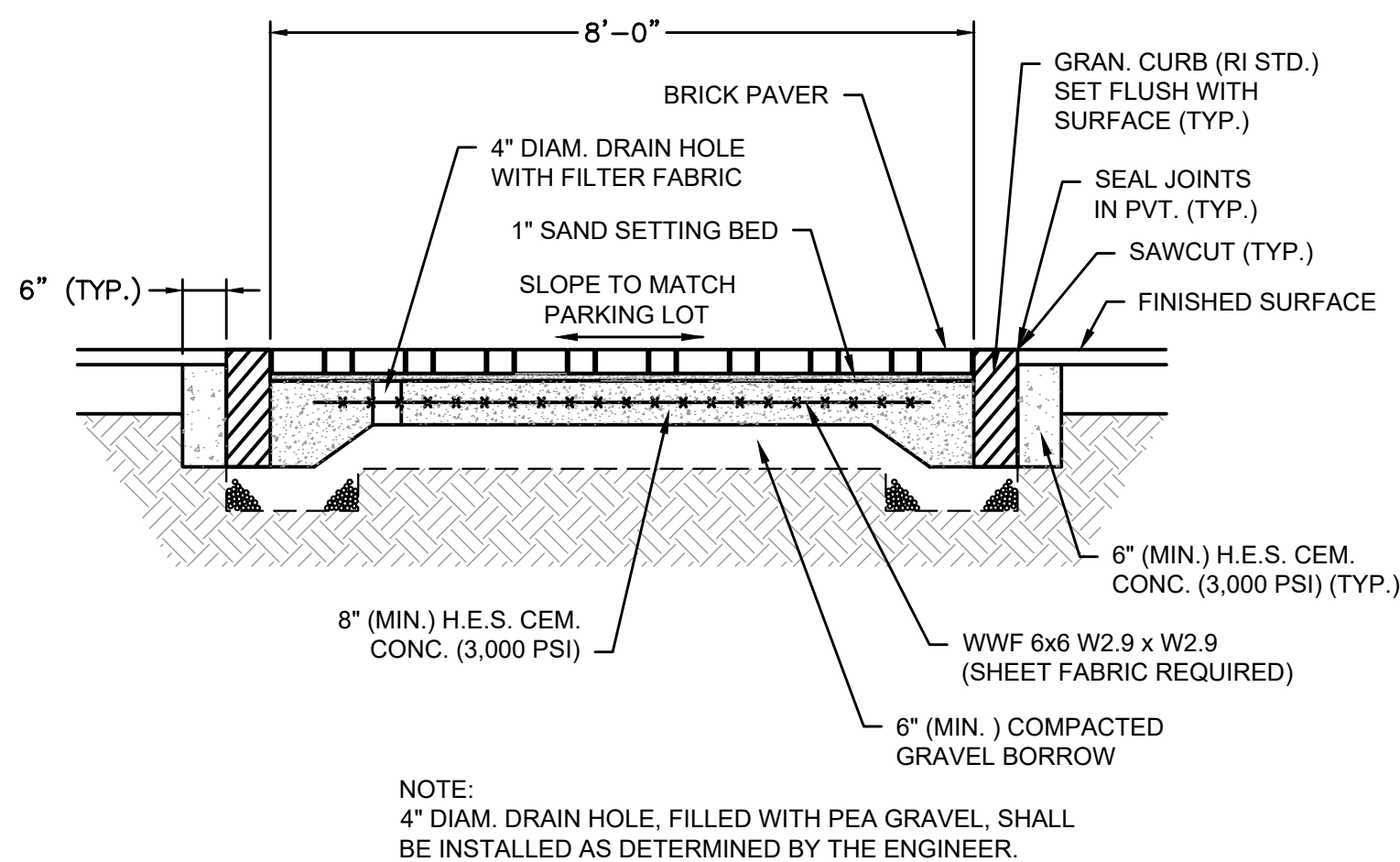
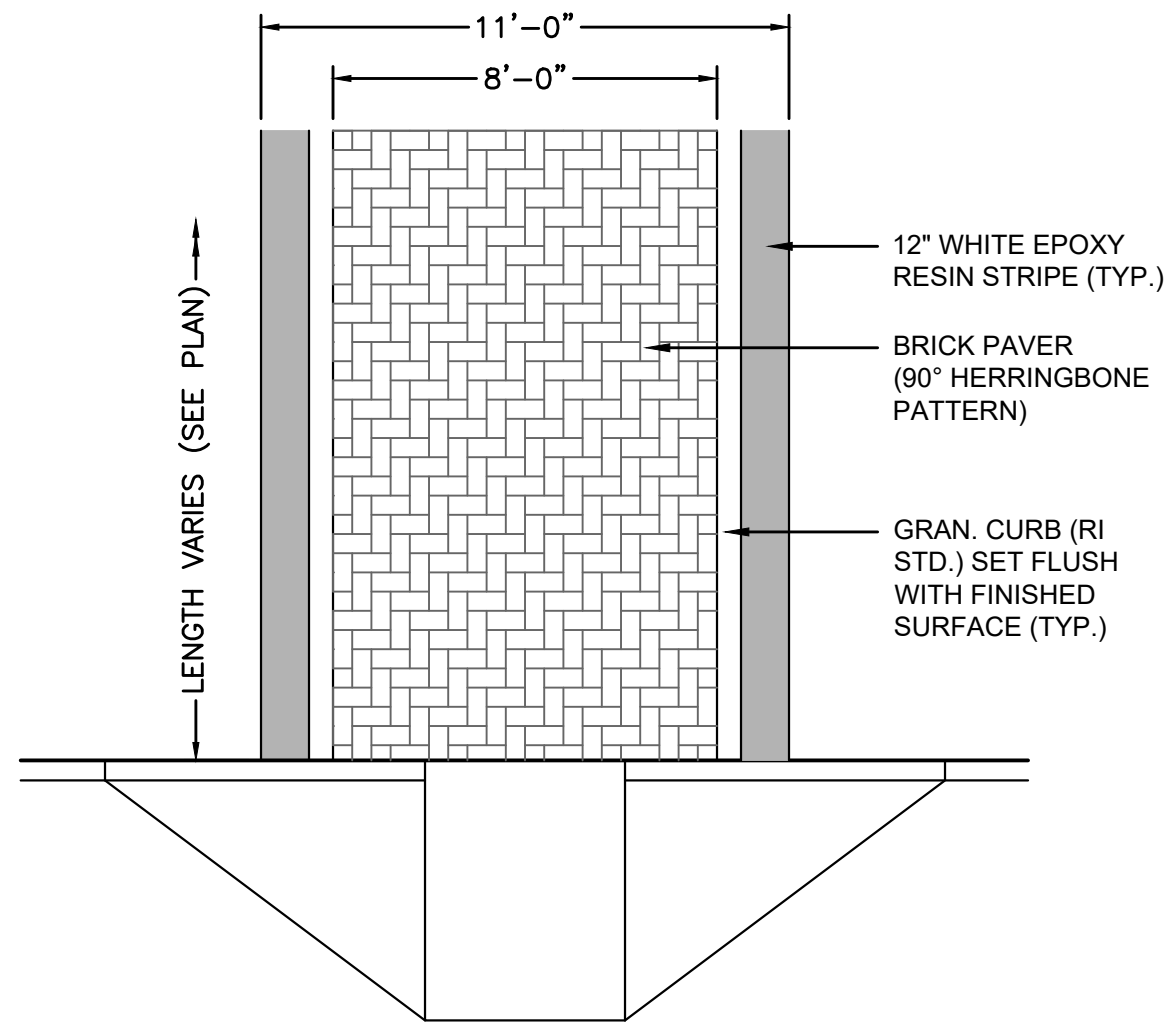
BETA JOB NO.: 7038

SCALE

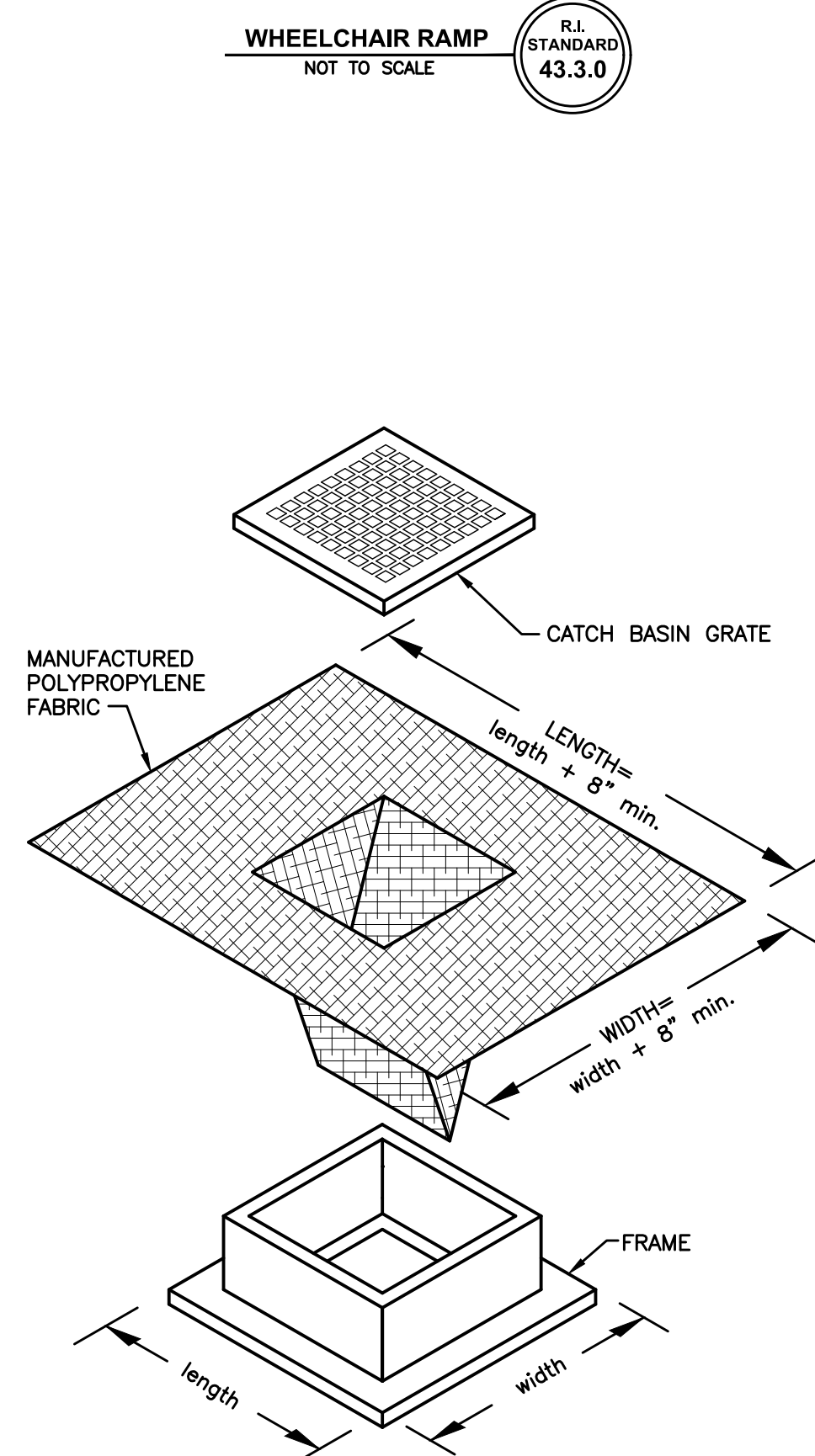
AS SHOWN

UNLESS OTHERWISE NOTED OR CHANGED BY REPRODUCTION

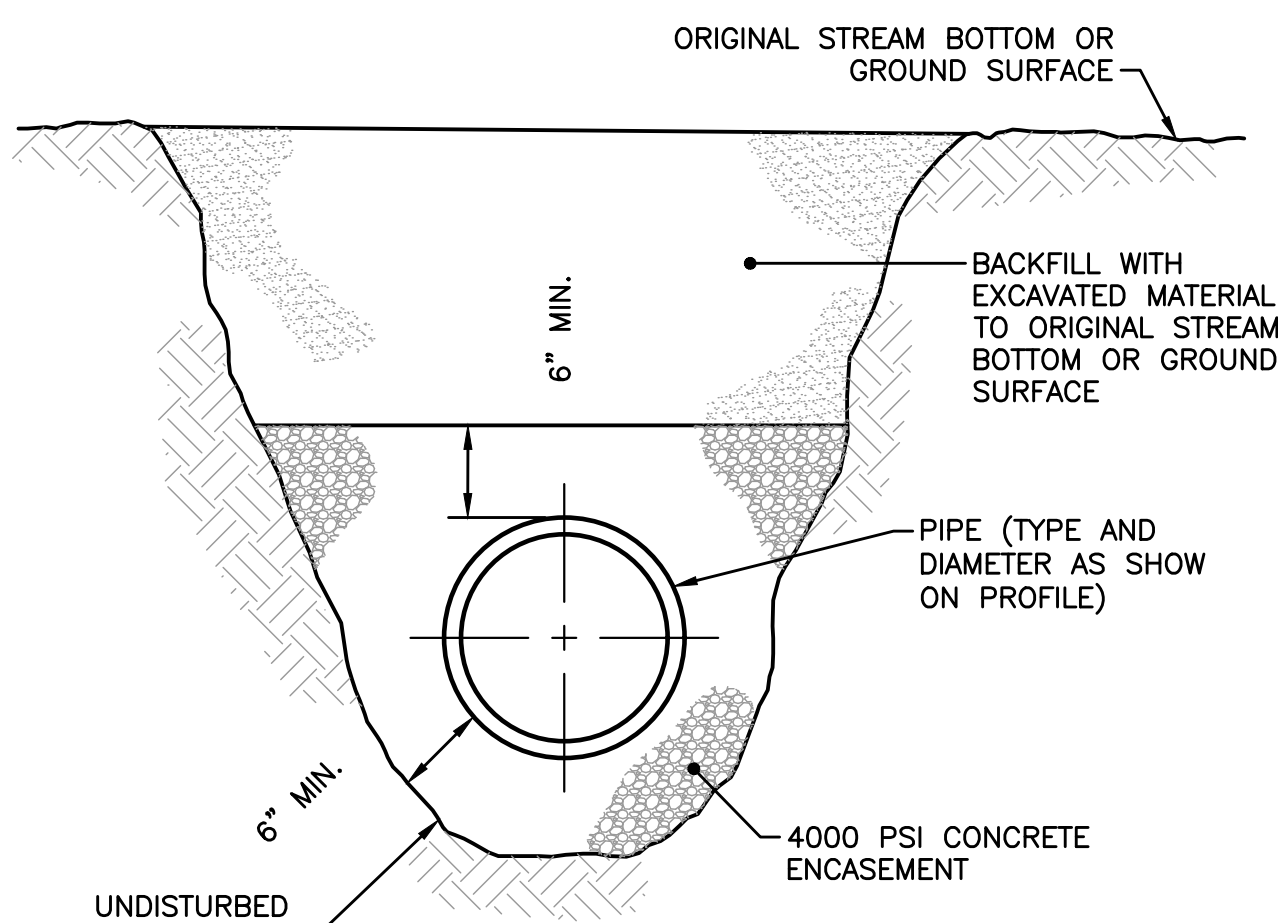
SHEET NO.



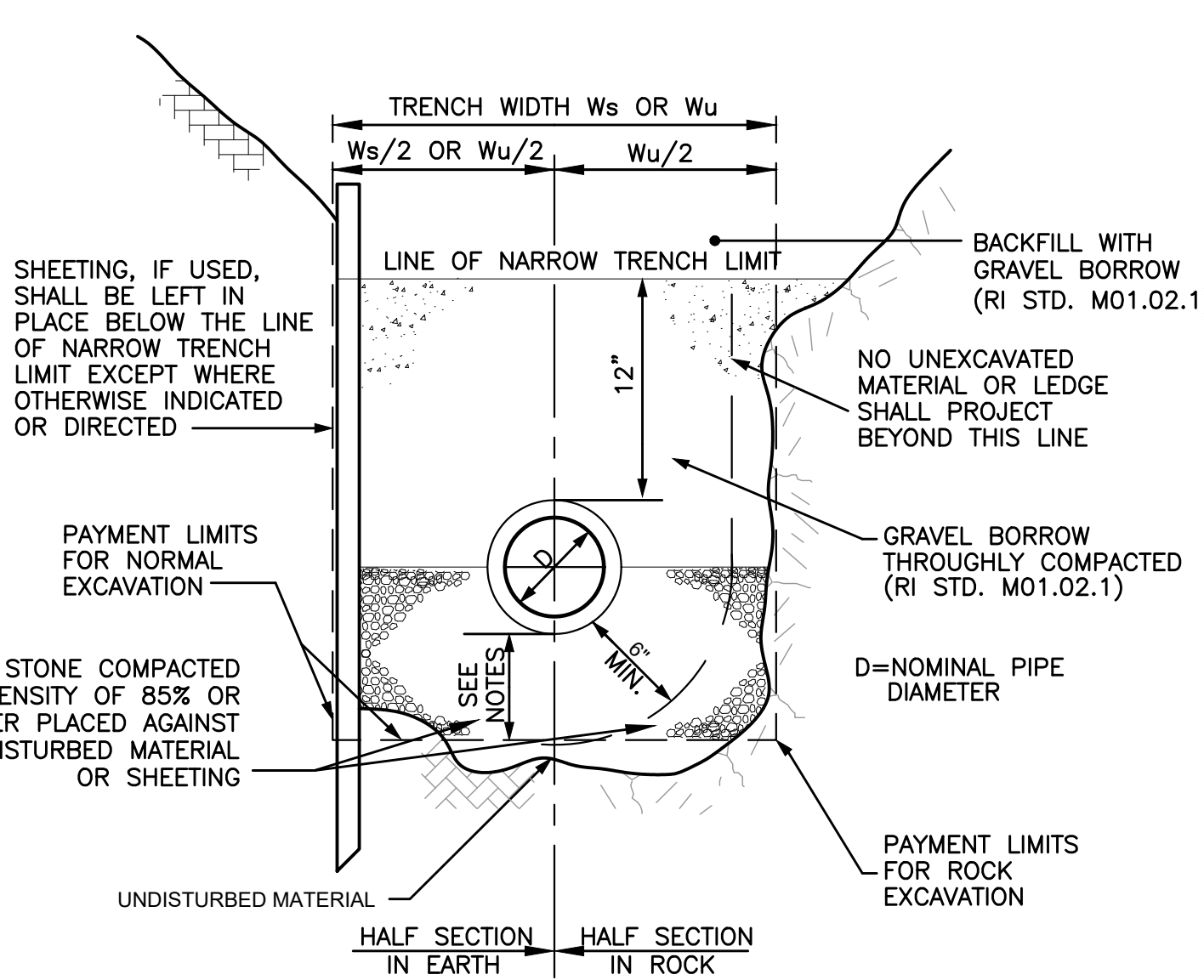
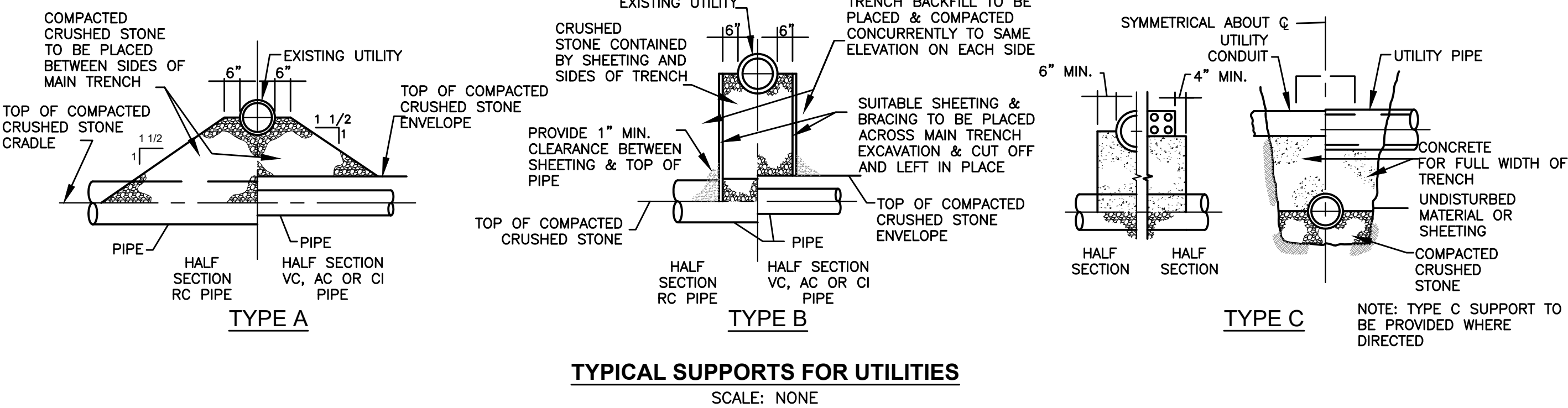
- NOTES:
1. SHALL BE IN ACCORDANCE WITH SECTION 905 OF THE R.I. STANDARD SPECIFICATIONS.
 2. WHEN ANY OBSTRUCTION LOCATED IN THE SIDEWALK FALLS WITHIN THE WHEELCHAIR RAMP WILL BE PLACED SUCH THAT THE OBSTRUCTION FALLS OUTSIDE OF THE RAMP.
 3. WHEN ANY OBSTRUCTION LOCATED IN THE SIDEWALK FALLS WITHIN THE WHEELCHAIR RAMP, THE WHEELCHAIR RAMP WILL BE PLACED SUCH THAT THE OBSTRUCTION FALLS OUTSIDE OF THE CROSSWALK, AND IT IS TO BE CENTERED WHENEVER POSSIBLE.
 4. DRAINAGE FACILITIES ARE TO BE LOCATED UNDER GRADE OF ALL WHEELCHAIR RAMPS.
 5. WHEELCHAIR RAMPS IS AS SHOWN ON CONTRACT DOCUMENTS.
 6. IN NO INSTANCE SHALL THE SIDEWALK CROSS SLOPE EXCEED 1:50 EXCEPT WITHIN THE RAMP AREA.
 7. IN NO INSTANCE SHALL THE SIDEWALK CROSS SLOPE BE LESS THAN A MINIMUM OF 1:50.
 8. THE WHEELCHAIR RAMP SLOPE AND SIDE SLOPES (TRANSITIONS), MUST NOT BE STEEPER THAN 1:12. HOWEVER, THESE SLOPES ARE FLATTER THAN WHEN SLOPES ARE LOCATED IN THE CROSSWALK.
 9. WHERE THE RAMP EXCEEDS THE HIGH SIDEWALK TRANSITION LENGTH, IT SHALL BE EIGHTEEN FEET (16'-0").
 10. IN NO CASE, WHERE A STOP LINE IS WARRANTED, SHALL A RAMP BE PLACED BEHIND THE STOP LINE.
 11. THE WHEELCHAIR RAMP SHALL BE CENTERED BEHIND THE STOP LINE.
 12. THE WHEELCHAIR RAMP SHALL BE CENTERED RADUALLY, OPPOSITE THE RADUS POINT WHEN POSSIBLE.
 13. WHEN A STOP LINE IS NOT WARRANTED, THE WHEELCHAIR RAMP SHALL BE CENTERED BEHIND THE STOP LINE.
 14. ALL REQUIRED CUTTING OF CURB PIECES TO BE PAID FOR UNDER COST OF CURB.
 15. DETECTABLE WARNING IS PAID FOR UNDER SECTION 942 OF THE R.I. STANDARD SPECIFICATIONS.
 16. THE WHEELCHAIR RAMP SHALL BE PAID FOR UNDER THE USE OF THE STOP LINE (MID-CROSSWALK) LOCATIONS.



- NOTES
1. LENGTH AND WIDTH OF POLYPROPYLENE FABRIC MUST EXCEED EXISTING CATCH BASIN FRAME DIMENSIONS BY A MINIMUM OF 8".
 2. REMOVE CATCH BASIN GRATE AND INSTALL POLYPROPYLENE FABRIC OVER CATCH BASIN FRAME. REPLACE CATCH BASIN GRATE TO SECURE POLYPROPYLENE FABRIC IN PLACE.



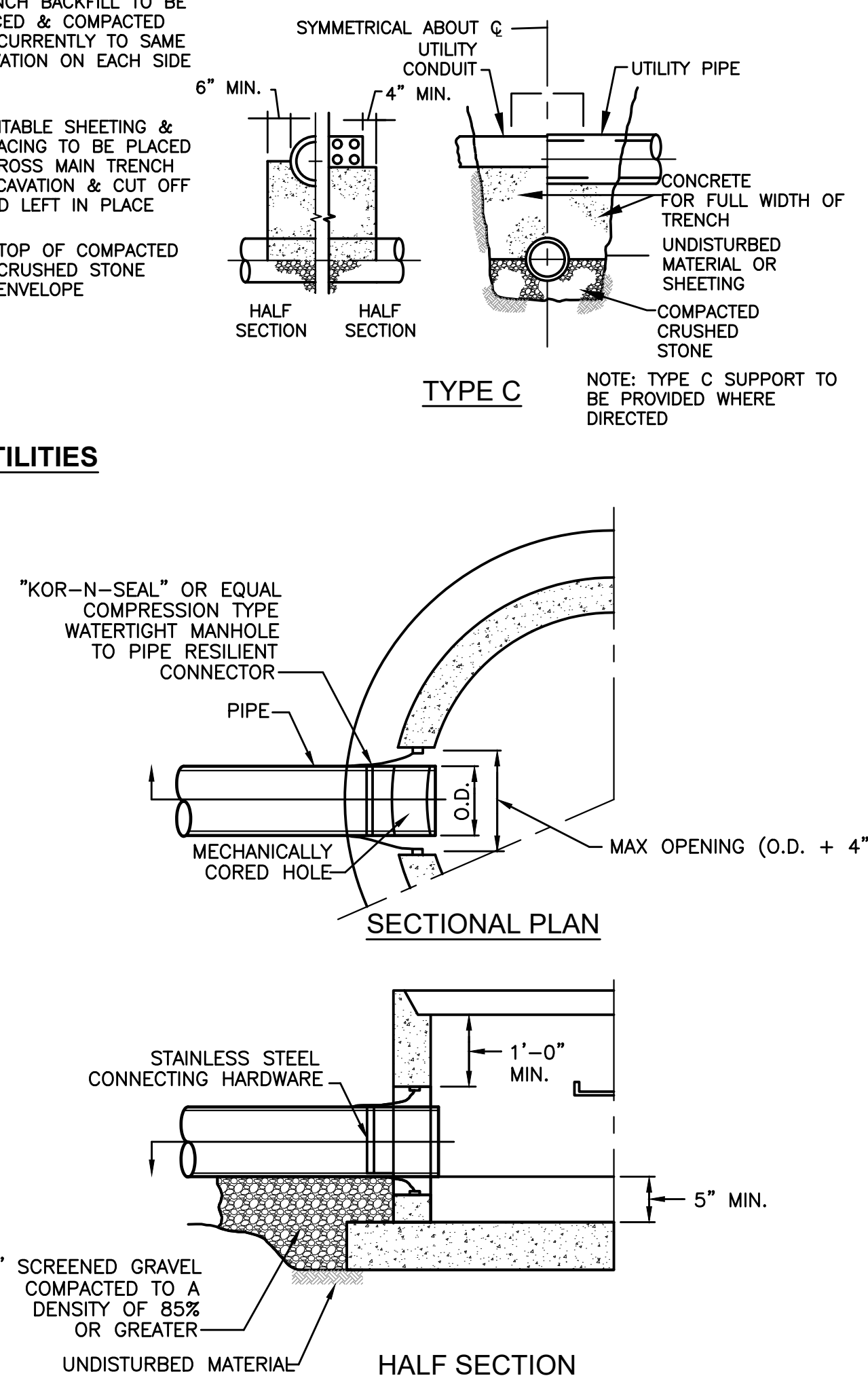
- NOTES**
1. THE PIPE SHALL BE PROPERLY SECURED TO PREVENT DISPLACEMENT DURING THE POURING OF CONCRETE ENCASEMENT.
 2. LIMIT OF CONCRETE ENCASEMENT SHALL BE SHOWN ON THE PROFILE OR AS DIRECTED.



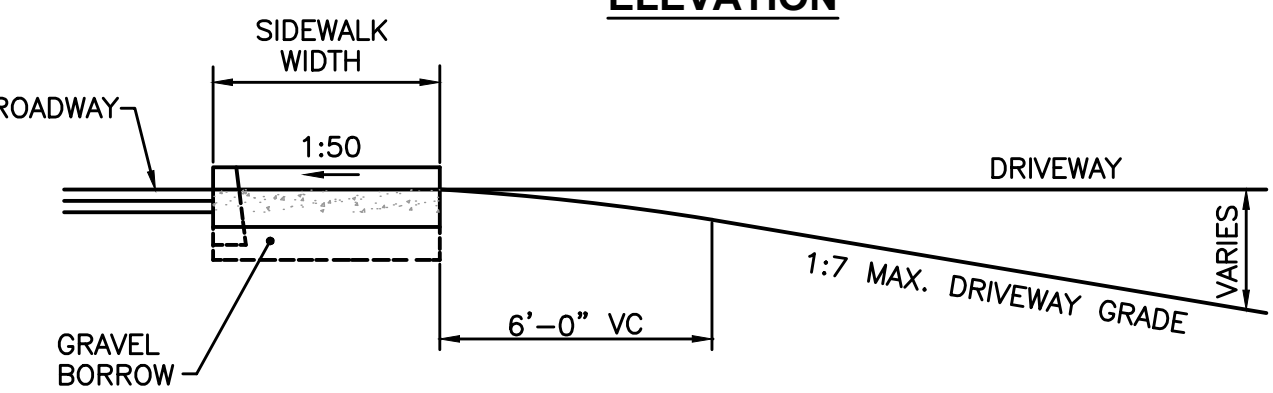
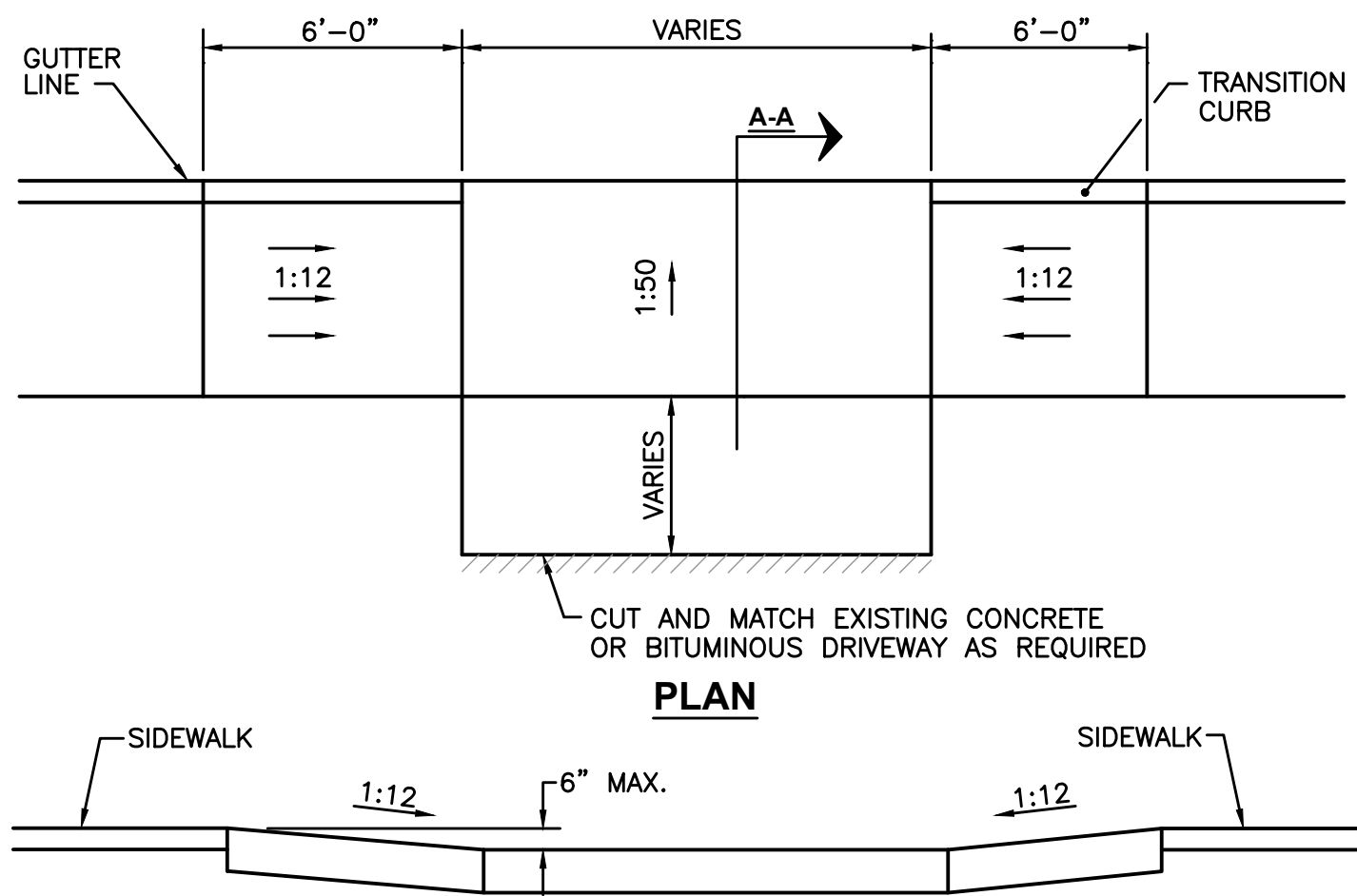
- NOTES:
1. IN STATE ROADS, THE BEDDING LIMIT FOR NORMAL EXCAVATION SHALL BE A MINIMUM DEPTH OF 12" BELOW DESIGN INVERT ELEVATIONS.
 2. IN TOWN ROADS, THE BEDDING LIMIT FOR NORMAL EXCAVATION SHALL BE A MINIMUM DEPTH OF 6" BELOW DESIGN INVERT ELEVATIONS.

MAXIMUM PAYMENT LIMITS						
DIAMETER OF PIPE D IN INCHES	TRENCH WIDTH IN FEET		TEMPORARY TRENCH PAVEMENT WIDTH IN FEET		PERMANENT TRENCH PAVEMENT WIDTH IN FEET*	
	TRENCH DEPTH		TRENCH DEPTH		TRENCH DEPTH	
	< OR = 10'	> 10' TO 20'	< OR = 10'	> 10' TO 20'	< OR = 10'	> 10' TO 20'
12 AND SMALLER	5.00	6.00	6.00	7.00	8.00	9.00
15	5.25	6.25	6.25	7.25	8.25	9.25
18	5.50	6.50	6.50	7.50	8.50	9.50
21	5.75	6.75	6.75	7.75	8.75	9.75
24	6.00	7.00	7.00	8.00	9.00	10.00
27	6.25	7.25	7.25	8.25	9.25	10.25
30	6.50	7.50	7.50	8.50	9.50	10.50
36	7.00	8.00	8.00	9.00	10.00	11.00
42	7.50	8.50	8.50	9.50	10.50	11.50
48	8.00	9.00	9.00	10.00	11.00	12.00
54	8.50	9.50	9.50	10.50	11.50	12.50
60	9.00	10.00	10.00	11.00	12.00	13.00
66	9.50	10.50	10.50	11.50	12.50	13.50
72	10.00	11.00	11.00	12.00	13.00	14.00

- NOTES
1. PERMANENT TRENCH PAVEMENT INCLUDES 1' CUT BACK OF TEMPORARY PAVEMENT ALONG EACH SIDE OF THE TRENCH
 2. TRENCH DEPTH MEASURED FROM THE EXISTING GROUND SURFACE TO 6" BELOW THE BOTTOM OF THE CONSTRUCTED PIPE.
 3. QUANTITIES FOR PAYMENT SHALL BE IN ACCORDANCE WITH THE ABOVE LIMITS OR THE ACTUAL WIDTHS, WHICHEVER IS LESS.



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NOTE:
SHALL BE IN ACCORDANCE WITH SECTION 904 OF THE R.I. STANDARD SPECIFICATIONS.

DRIVEWAY DEVELOPMENT FOR 6'-0" TRANSITION CURB

NOT TO SCALE

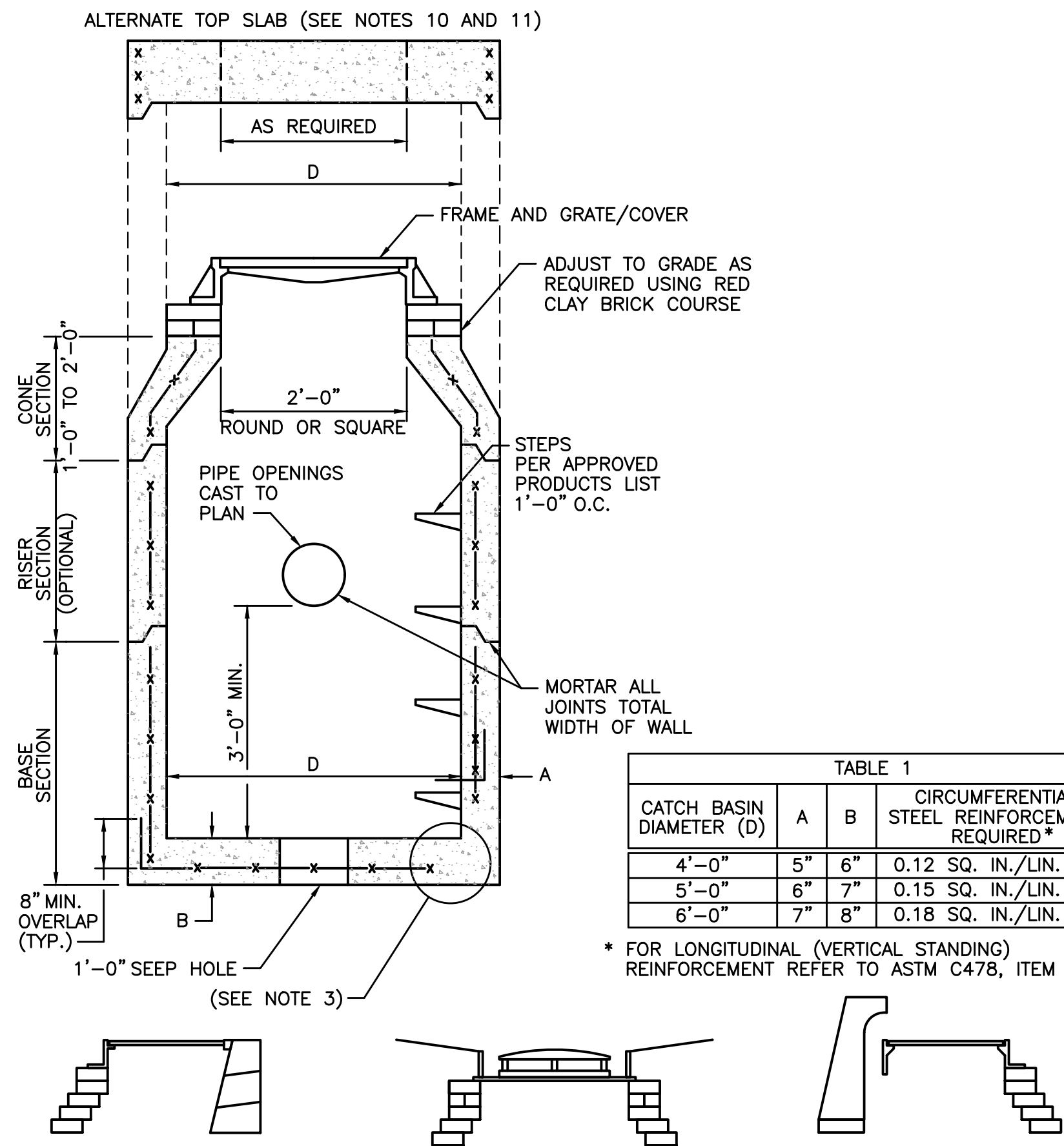


TABLE 1		CIRCUMFERENTIAL STEEL REINFORCEMENT REQUIRED *	
CATCH BASIN DIAMETER (D)	A	B	
4'-0"	5"	6"	0.12 SQ. IN./LIN. FT.
5'-0"	6"	7"	0.15 SQ. IN./LIN. FT.
6'-0"	7"	8"	0.18 SQ. IN./LIN. FT.

* FOR LONGITUDINAL (VERTICAL STANDING)
REINFORCEMENT REFER TO ASTM C478, ITEM 8.1.2

TYPE "D" TYPE "R" TYPE "F" TYPE CATCH BASIN AS REQUIRED

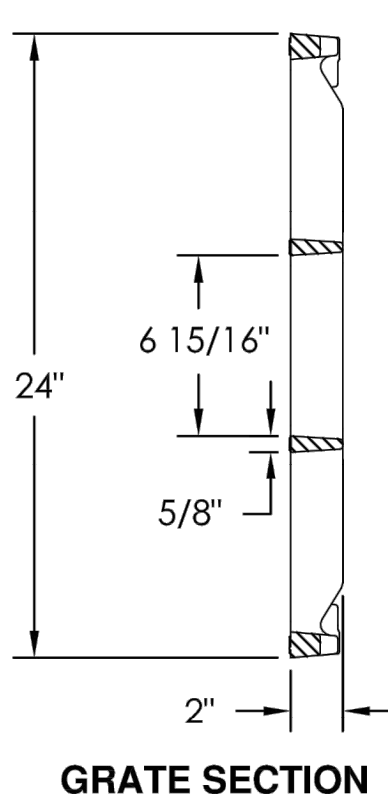
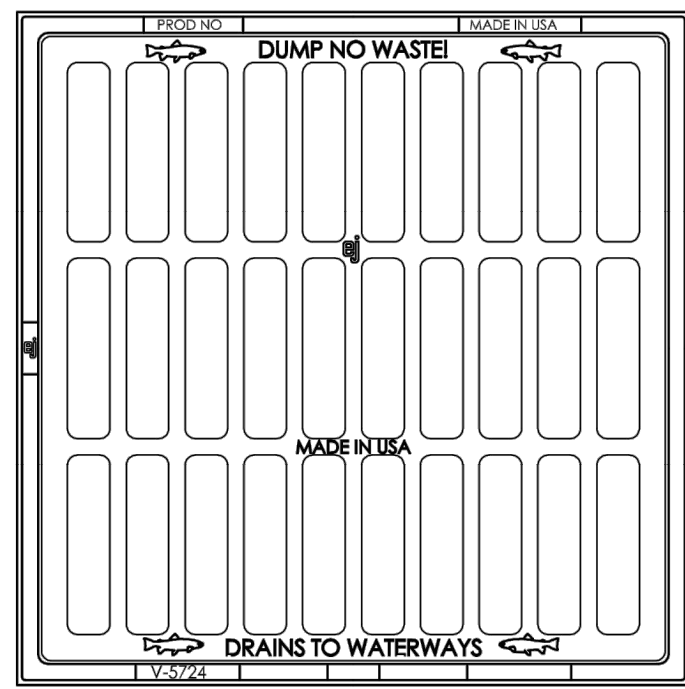
- NOTES:
- SHALL BE IN ACCORDANCE WITH SECTION 702 OF THE R.I. STANDARD SPECIFICATIONS.
 - SEE TABLE 1 FOR STEEL REINFORCEMENT REQUIREMENTS.
 - STEEL REINFORCEMENT FOR BASE SECTION BOTTOM SHALL BE A MINIMUM OF 0.12 SQ. IN./LIN. FT. (BOTH WAYS).
 - STEPS SHALL CONFORM TO STD. 5.3.0 AND SHALL BE INSTALLED AT THE CASTING POINT.
 - ONE POUR MONOLITHIC BASE SECTION.
 - ANY NECESSARY ADJUSTMENTS DURING CONSTRUCTION WILL BE DONE BY SAW-CUTTING AND/OR CORING ONLY. NO JACKHAMMERS, HAMMERS AND CHISELS OR PNEUMATIC TOOLS WILL BE ALLOWED.
 - CORBEL MADE OF RED CLAY BRICK WILL BE PERMITTED FOR THE "CONE SECTION" OF THE 4'-0" CATCH BASIN ONLY.
 - FOR CATCH BASIN TYPES "D" AND "F" STEPS MUST BE INSTALLED ON THE CURB SIDE OF THE STRUCTURE.
 - THE CENTERLINE OF THE OPENING MUST BE WITHIN 2'-0" FROM THE STEPS.
 - ALTERNATE TOP SLAB IS STEEL REINFORCED TO MEET OR EXCEED H-25 LOADING (SEE STD. 4.7.2).
 - ALTERNATE TOP SLAB IS ONLY FOR USE WHEN REDUCING SECTION DOES NOT FIT BECAUSE OF STRUCTURE DEPTH.
 - REFER TO STD. 5.2.0 FOR MAXIMUM PIPE SIZES.
 - PROVIDE A MINIMUM OF 12 INCHES OF COMPACTED GRAVEL BORROW AROUND THE STRUCTURE.

PRECAST 4'-0", 5'-0", OR 6'-0" ROUND CATCH BASIN

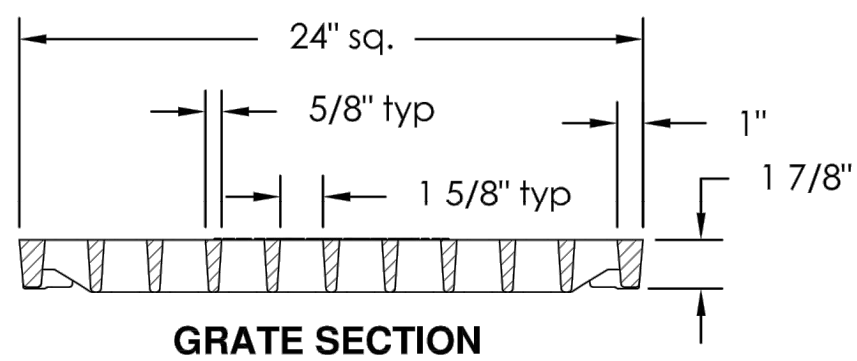
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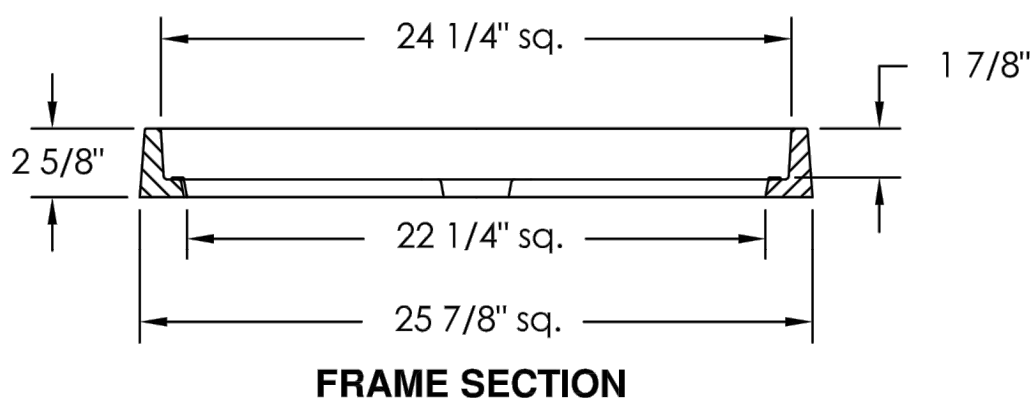
V5724 V5724-4 Assembly



GRATE SECTION



GRATE SECTION



FRAME SECTION

LOW PROFILE FRAME AND GRATE

NOT TO SCALE



Product Number

45724007

Design Features

- Materials
- Frame
- Gray Iron (CL35B)
- Grate
- Ductile Iron (70-50-05)

-Design Load

- Heavy Duty
- 60,000 Lbs. Proof Load Rated
- Open Area
- 336 sq in
- Coating
- Undipped
- ~√ Designates Machined Surface

Weight

153 lb

Certification

- ASTM A536
- ASTM A48
- Country of Origin: USA

Major Components

45724010

45724431

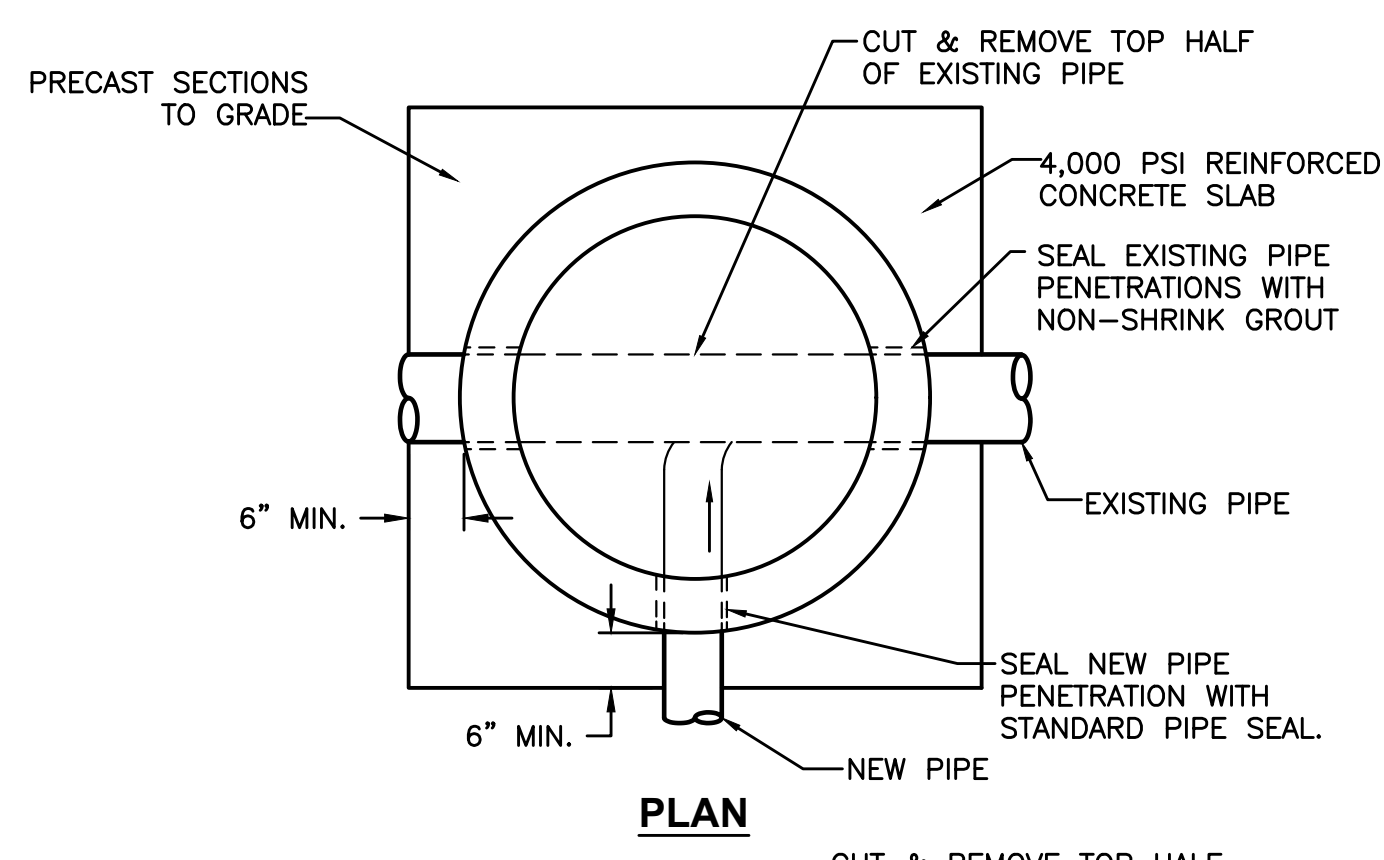
Drawing Revision

9/11/2008 Designer: GAD

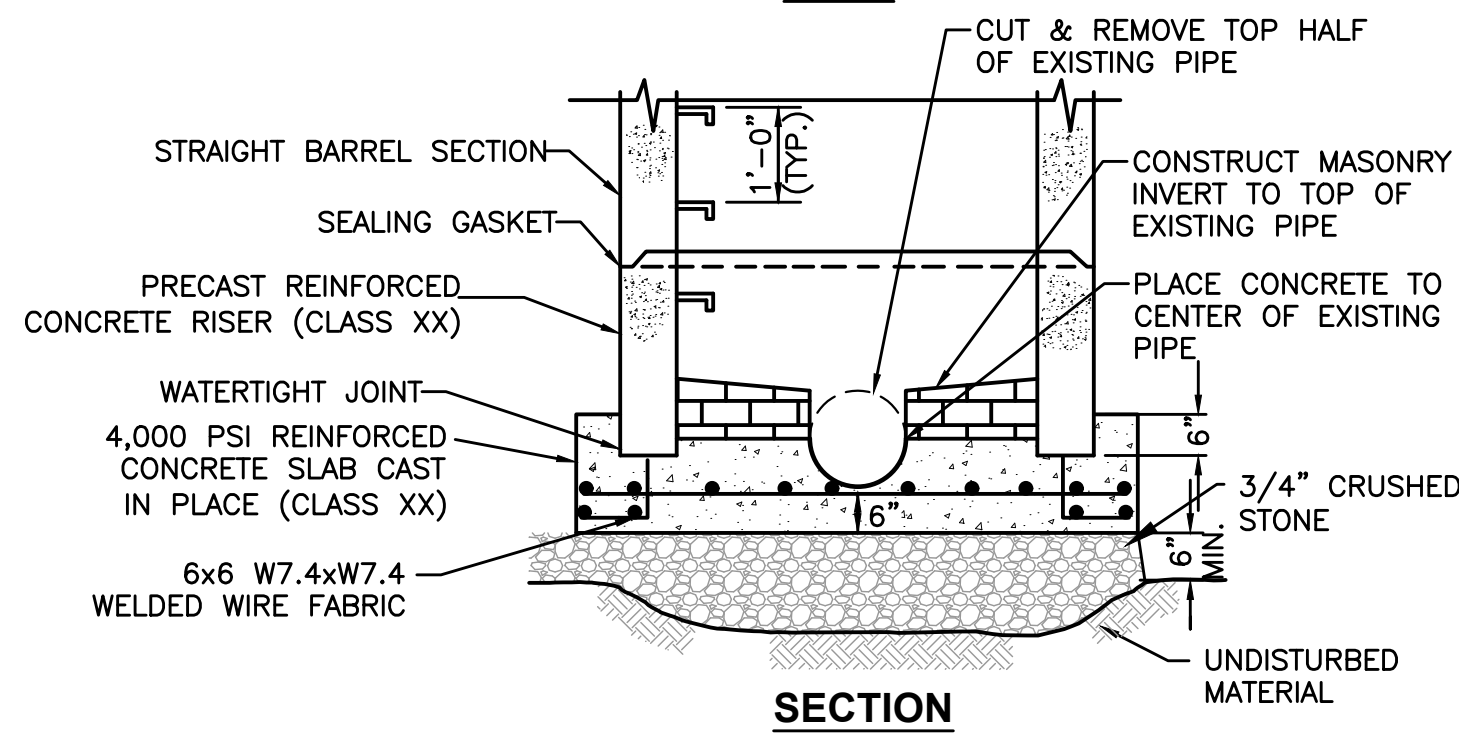
4/20/2016 Revised By: MAH

Disclaimer

Weights (ba Ag) dimensions (inches/mm) and drawings provided for your guidance. We reserve the right to modify specifications without prior notice.
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PLAN



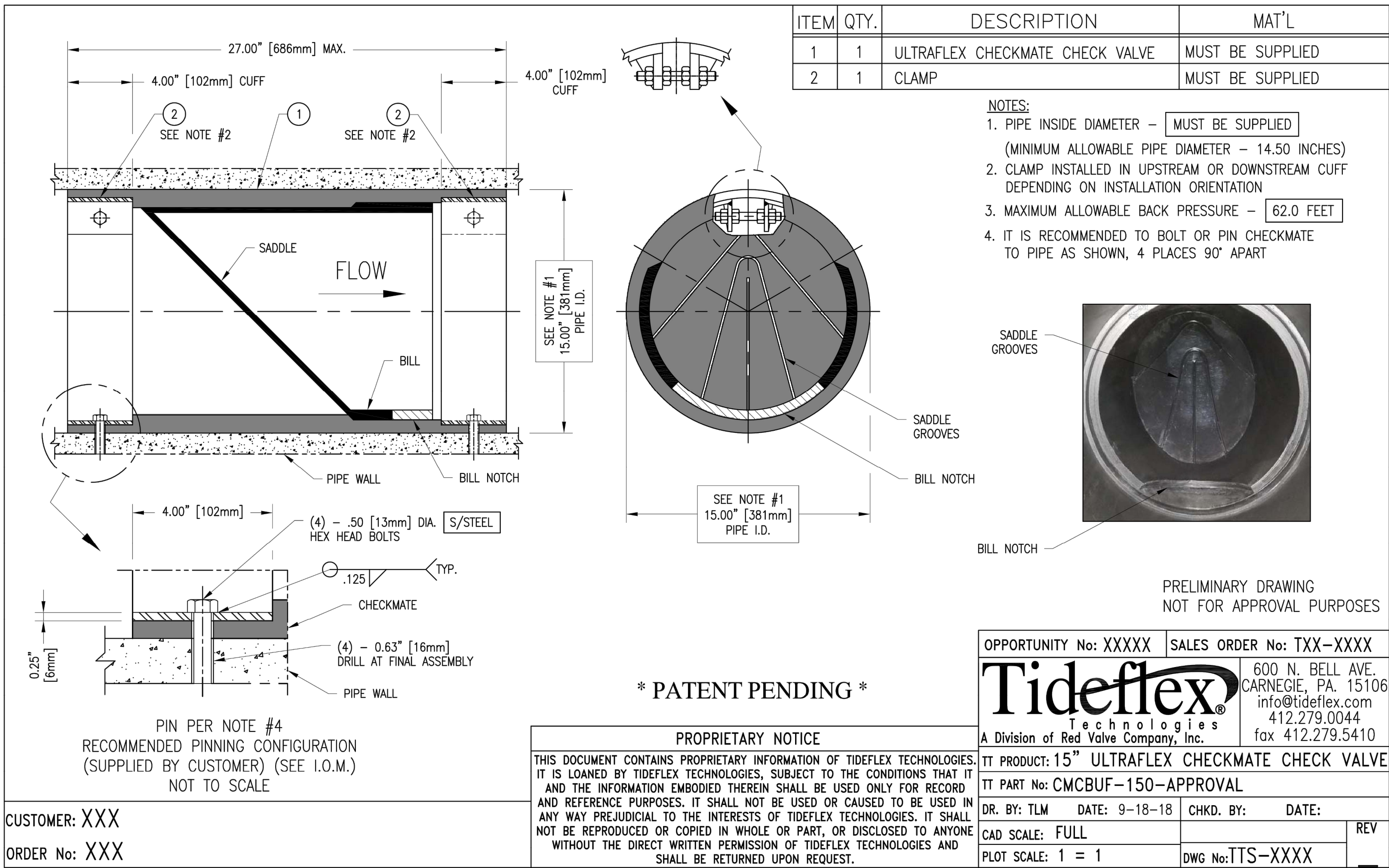
SECTION

NOTES:

- FOR MANHOLES IN LOCATIONS REQUIRING CONTROLLED DENSITY FILL, PAYMENT LIMITS SHALL BE A MAXIMUM OF 2.25 CY PER VERTICAL FOOT OF MANHOLE.
- SEE SPECIFICATION SECTION 02607 FOR MANHOLE COMPONENT REQUIREMENTS.
- SEE SPECIFICATION SECTION 03300 FOR CONCRETE REQUIREMENTS.
- CONSTRUCT TO SIZES (4' OR 5') AS INDICATED ON PLANS.

TYPICAL "DOGHOUSE" MANHOLE OVER EXISTING PIPE

NOT TO SCALE



OPPORTUNITY No: XXXXX SALES ORDER No: TXX-XXXX

Tideflex Technologies
A Division of Red Valve Company, Inc.
600 N. BELL AVE.
CARNEGIE, PA. 15106
info@tideflex.com
412.279.0044
fax 412.279.5410

TT PRODUCT: 15" ULTRAFLEX CHECKMATE CHECK VALVE

TT PART No: CMCBUF-150-APPROVAL

DR. BY: TLM DATE: 9-18-18 CHKD. BY: DATE:

CAD SCALE: FULL

PLOT SCALE: 1 = 1 DWG No: TTS-XXXX

PROPRIETARY NOTICE

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TIDE GATE DETAIL (OR APPROVED EQUAL)

NOT TO SCALE

PREPARED BY



REGISTERED PROFESSIONAL



SUBCONSULTANT

PROJECT

Wickford Waterfront Improvements

Wickford, RI

TITLE

CONSTRUCTION DETAILS 4

NO. REVISIONS DATE

DRAWN BY: CC

DESIGNED BY: TD

CHECKED BY: NBI

ISSUE DATE: 4/17/2024

BETA JOB NO.: 7038

SCALE

AS SHOWN

UNLESS OTHERWISE NOTED OR CHANGED BY REPRODUCTION

SHEET NO.

NICOLE B. IANNUZZI

No.  9724

REGISTERED
PROFESSIONAL ENGINEER
(CIVIL) 4-17-24

PROJECT 1

Wickford, RI

CONSTRUCTION DETAILS 5

NO.	REVISIONS	DATE

DRAWN BY:	CC
DESIGNED BY:	TD
CHECKED BY:	NBI
ISSUE DATE:	4/17/2024
BETA JOB NO.:	7038

AS SHOWN

UNLESS OTHERWISE NOTED OR CHANGED BY REPRODUCTION

20



RAIN GUARDIAN
FORTRESS
PRETREATMENT
CHAMBER

RAIN GUARDIAN
FORTRESS
PRETREATMENT
CHAMBER

**DO NOT LIFT FORTRESS
BY METAL FACE PLATES**



**LIFT ONLY BY CABLES
WITH SPREADER BAR**

- 1. STEEL REINFORCED, COLD JOINT SECURED MONOLITHIC CONCRETE STRUCTURE (3.26 CF CONCRETE, 499 LBS, 1.36 CF STORAGE VOLUME), CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,500 PSI AT 28 DAYS. CONCRETE AIR ENTRAINED (5% TO 8.5% BY VOLUME). MANUFACTURED AND DESIGNED TO ASTM C858.**
- 2. TWO-POINT PICK (EMBEDDED LIFTING CABLES)**
- 3. INTERNAL GRATE - STEEL GRATE (25 LBS/PIECE, 1" THICK), 361 LB CONCENTRATED LOAD OR 206 LB/SQ-FT UNIFORM LOAD**
- 4. TOP LID - STEEL GRATE (27 LBS/PIECE, 1" THICK), 361 LB CONCENTRATED LOAD OR 206 LB/SQ-FT UNIFORM LOAD**
- 5. STEEL FACE PLATE (INLET AND OUTLET) - A36 1/4" THICKNESS**
- 6. REMOVABLE FILTER ASSEMBLY (FIBERGLASS GRATE WITH FILTER AND ALUMINUM CHANNEL)**

1. EXCAVATE AN AREA WITH DIMENSIONS OF AT LEAST 4'-6" L X 2'-0" W FOR THE RAIN GUARDIAN. THE DISTANCE FROM THE BACK OF THE CURB MAY VARY BASED ON SITE CONDITIONS, BUT CONSIDERATIONS SHOULD INCLUDE SLOPE OF THE INLET AND BASIN SIDE SLOPES ADJACENT TO THE RAIN GUARDIAN. POSITION RAIN GUARDIAN SO PRIMARY OUTLET ALIGNS WITH TOE OF BASIN SIDE SLOPE TO AVOID SOIL INTERFERENCE WITH REMOVABLE FILTER WALL. THE EXCAVATION DEPTH SHOULD BE 1'-7" BELOW THE GUTTERLINE ELEVATION AT THE RAIN GARDEN INLET TO ACCOMMODATE THE 6" AGGREGATE BASE, 4" RAIN GUARDIAN BASE, AND 9" PONDING DEPTH.
2. INSTALL AN AGGREGATE BASE, COMPACTED TO 95% STANDARD PROCTOR, WITH DIMENSIONS OF AT LEAST 4'-6" L X 2'-0" W X 6" D. FINISHED TOP ELEVATION OF AGGREGATE TO BE PRECISELY 1'-1" BELOW THE GUTTER LINE ELEVATION AT THE RAIN GARDEN INLET.
3. SET RAIN GUARDIAN ON THE PREPARED AGGREGATE BASE. THE RAIN GUARDIAN INLET WILL BE 1-1/2" BELOW THE GUTTERLINE ELEVATION TO ACCOMMODATE A SLOPED INLET FROM THE GUTTER TO THE RAIN GUARDIAN.
4. INSTALL FRAMING FOR INLET BETWEEN RAIN GUARDIAN AND BACK OF CURB. TOP ELEVATIONS OF THE FRAMING SHOULD MATCH THE TOP OF THE CURB ON THE STREET SIDE AND THE TOP OF THE RAIN GUARDIAN ON THE BIORETENTION SIDE.
5. INSTALL EXPANSION/CONTRACTION JOINT MATERIAL OR A SHEET OF POLY TO SERVE AS A BOND BREAK BETWEEN RAIN GUARDIAN AND CONCRETE INLET BEFORE POURING INLET.
6. SIDE CURBS OF THE POURED INLET MUST HAVE AN INSURMOUNTABLE PROFILE TO PREVENT WATER FLOW FROM OVERTOPPING THE DOWNSTREAM SIDE OF THE INLET.
7. INSTALL REMOVABLE FILTER. THE FILTER SHOULD BE INSTALLED IN THE CHANNEL AT THE RAIN GUARDIAN OUTLET WITH THE FILTER FABRIC FACING THE RAIN GUARDIAN INLET.
8. INSTALL INTERNAL GRATE AND THEN TOP LID.

REV	BY	DATE	DESCRIPTION
A	MDH	11/16/22	FORTRESS

SCALE	VARIABLE
U.S. PATENT NOS.	8,501,016 AND 8,858,804

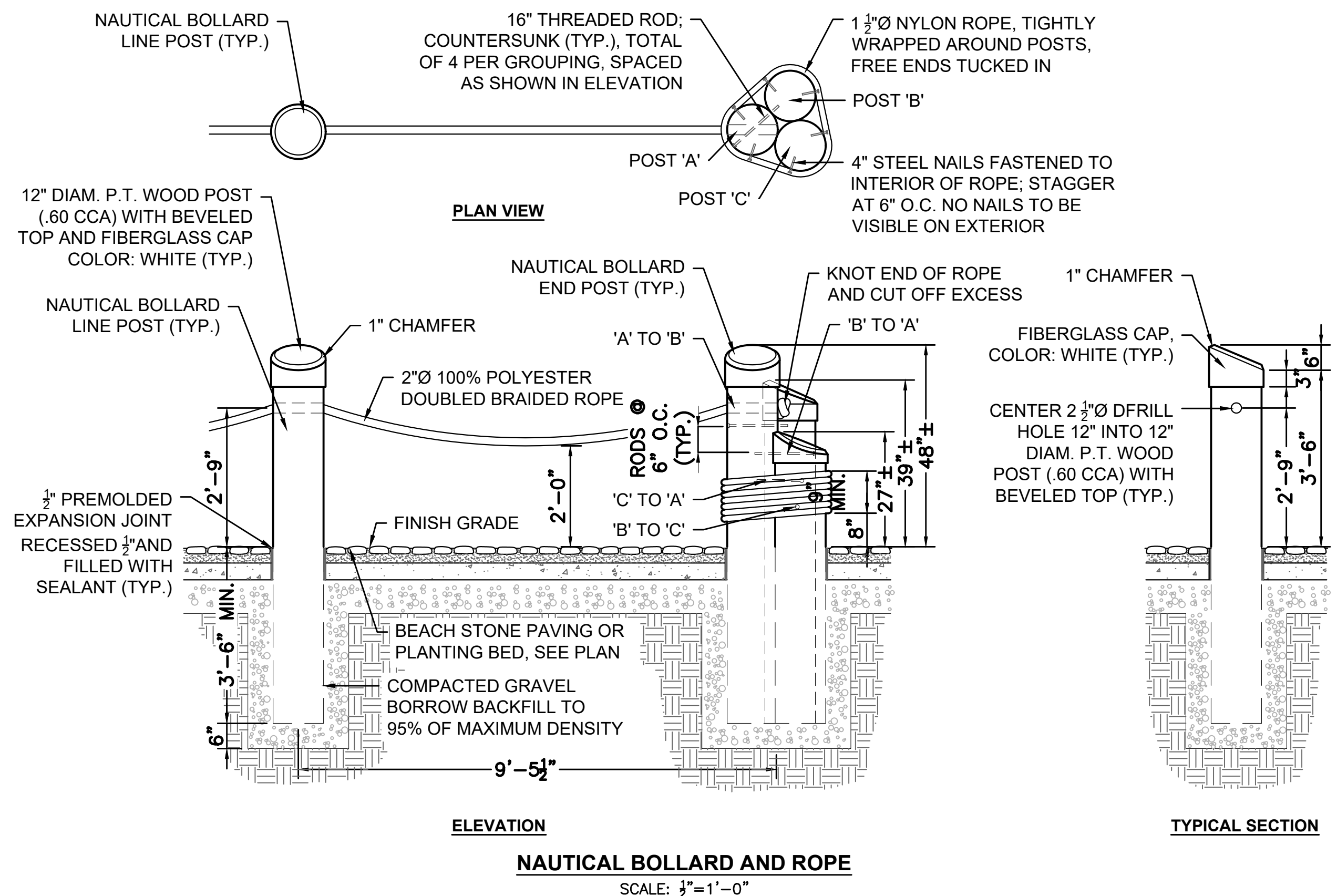
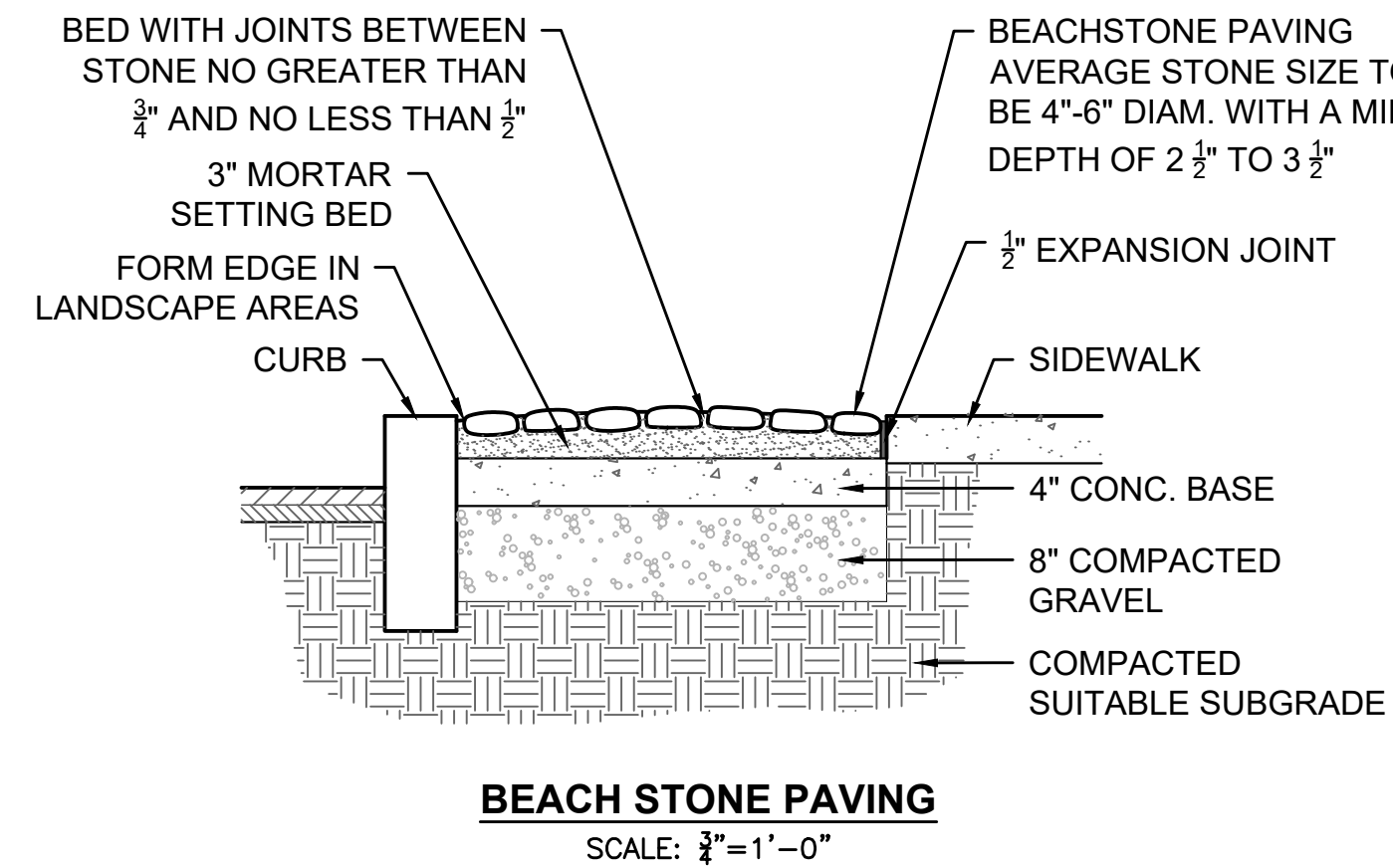
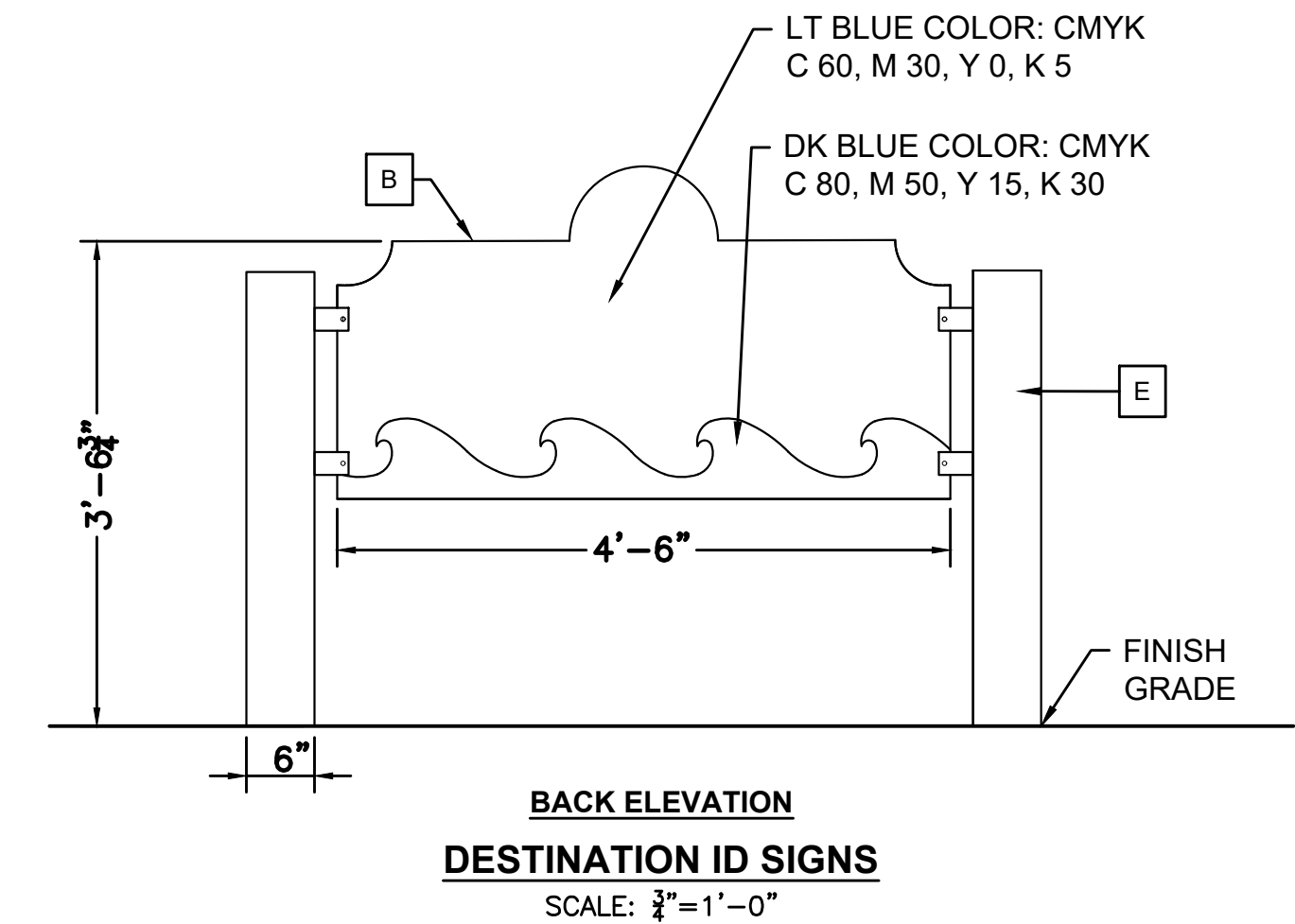
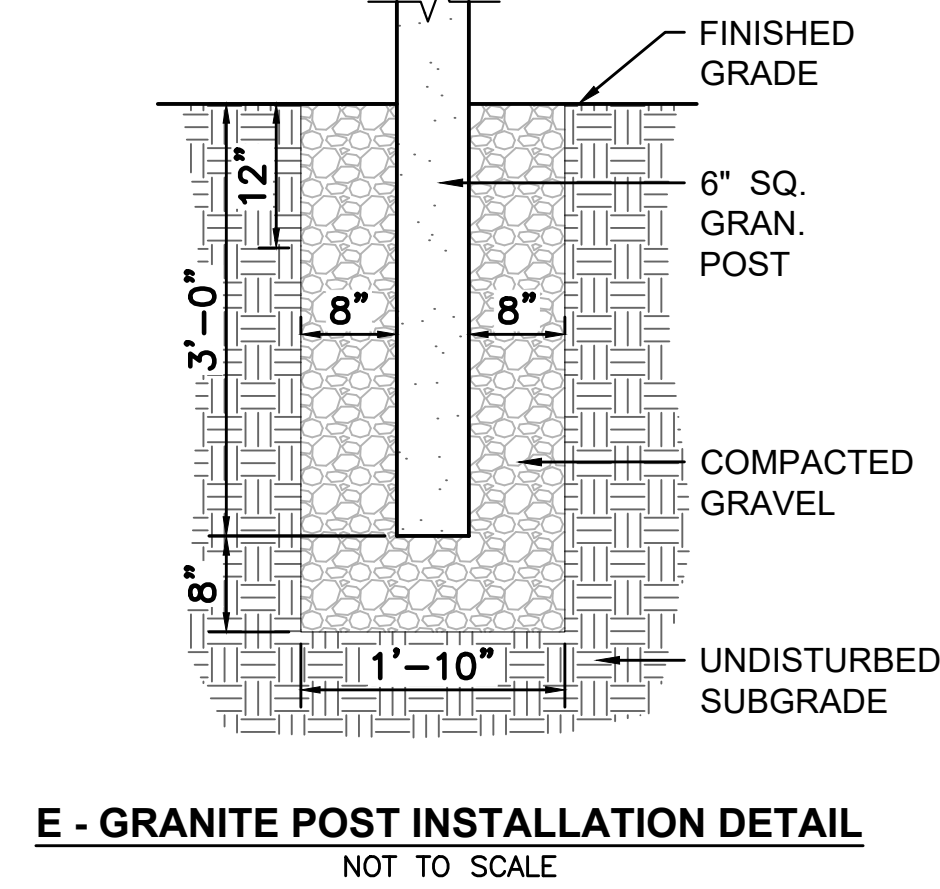
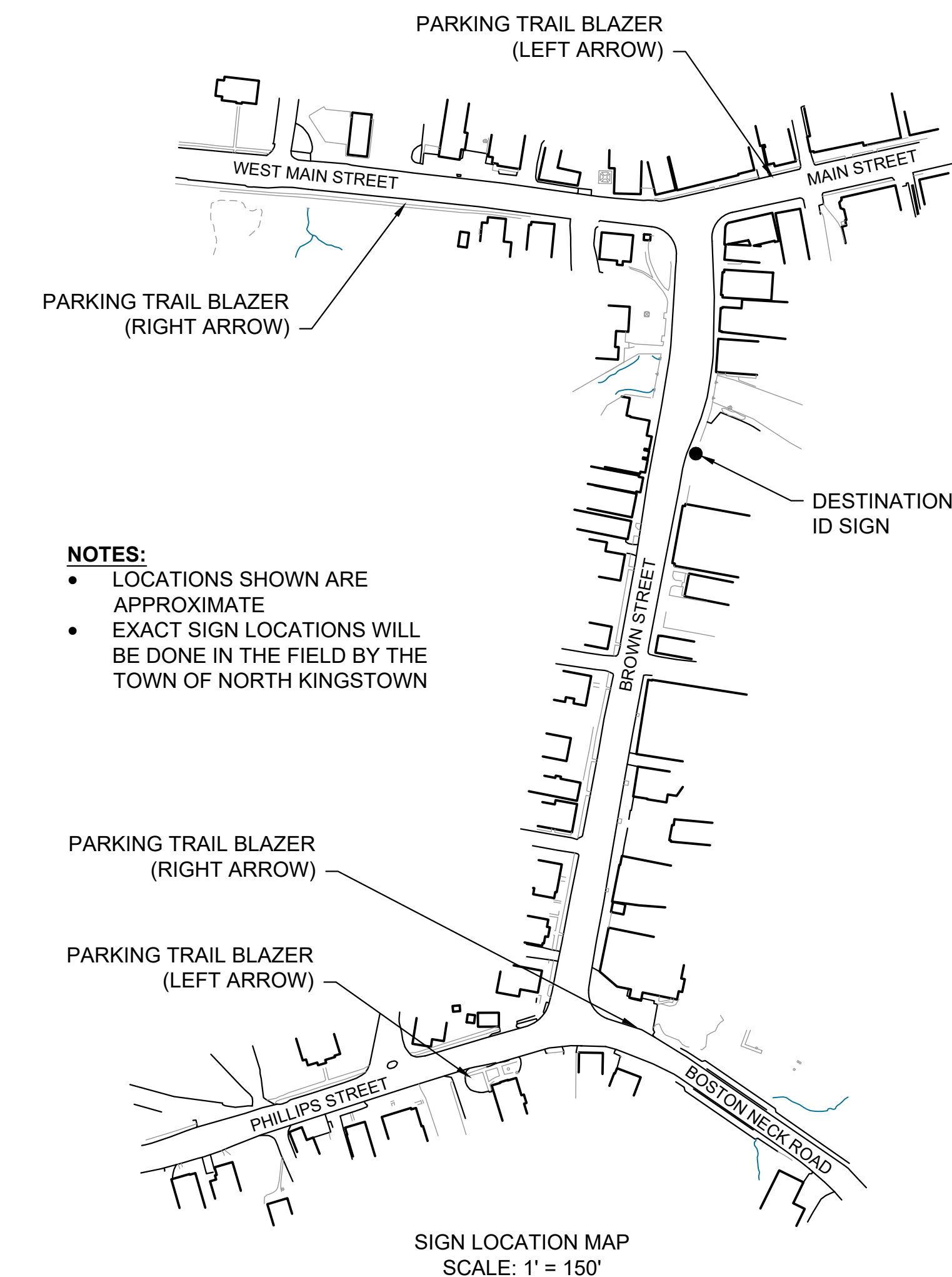


Stoneworks
ARCHITECTURAL PRECAST / CAST STONE

NATIONAL PRECAST CONCRETE ASSOCIATION
CERTIFIED PLANT



**RAIN GUARDIAN FORTRESS
PRETREATMENT CHAMBER
BIORETENTION PONDING DEPTH: 9"
TYPICAL DETAIL**



- NOTES:**
- A. SQUARE POST (SIZE AS NOTED)**
- MATERIAL: ALUMINUM SQUARE TUBE 6061 - SIZE AS NOTED, .250" THICK WALL
 - FABRICATION PROCESS: EXTRUDED
 - CORNERS: SQUARE, SMOOTH
 - COLOR: CUSTOM, AS NOTED
 - SURFACE PROCESS: PAINT ALL EXPOSED SURFACES WITH MATHEWS ACRYLIC POLYURETHANE, WITH CLEAR COAT SATIN FINISH.
- B. 1/4" SIGN PANEL FRONT - REFLECTIVE**
- MATERIAL: $\frac{1}{4}$ " THK. ALUMINUM SHEET
 - FABRICATION PROCESS: ROUTER CUT SMOOTH
 - COLORS: AS NOTED
 - LT BLUE COLOR: CMYK: C 60, M 30, Y 0, K 3
 - DK BLUE COLOR: CMYK: C 80, M 50, Y 15, K 50
 - GRAPHIC BACKGROUND AND COPY PROCESS: 3M CUSTOM INKS PRINT DIRECT TO 3830 WITH 3M APPROVED UV/GRAFFITI VINYL OVER-LAMINATE.
 - FASTENER: PRESS-ROLL PER 3M REQUIREMENTS, *MUST COMPLY WITH MUTCD SECTION TABLE 2A-3; MINIMUM MAINTAINED RETRO-REFLECTIVITY LEVELS.
- C. SIGN PANEL BRACKET**
- PROVIDE MECHANICAL FASTENERS PER POST (AS NOTED)
 - MATERIAL: ALUMINUM ANGLE - .250" WALL
 - FABRICATION PROCESS: EXTRUDED, SAW CUT
 - EDGES: SQUARE, SMOOTH
 - COLOR: CUSTOM, AS NOTED
 - SURFACE PROCESS: PAINT ALL EXPOSED SURFACES WITH MATHEWS ACRYLIC POLYURETHANE, WITH CLEAR COAT SATIN FINISH.
 - FASTENER: TAMPER-RESISTANT MECHANICALLY FASTENED TO POST. WELD TO PANEL.
- D. ART PATTERNS (AS REQUIRED)**
- COLOR: CUSTOM
 - GRAPHICS APPLICATION: APPLIED VINYL
 - SURFACE PROCESS: GRAPHIC BACKGROUND AND COPY PROCESS: 3M CUSTOM INKS PRINT DIRECT TO VINYL WITH 3M APPROVED UV/GRAFFITI VINYL OVER-LAMINATE.
- E. 6" SQUARE GRANITE POSTS.**

3. ALL PLANS INDICATED ON THESE PLANS SHALL BE IN ACCORDANCE WITH:
 - THE 2018 EDITION OF AND SUPPLEMENTS TO THE RHODE ISLAND DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION (RI STANDARD SPECIFICATIONS).
 - THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) LRFD BRIDGE CONSTRUCTION SPECIFICATIONS, 8th EDITION, NOVEMBER 2017, INCLUDING THE LATEST INTERIM REVISIONS.
2. DIMENSIONS, STATIONS, AND ELEVATIONS ARE SHOWN TO THE NEAREST ONE-HUNDREDTH OF A FOOT OR ONE-EIGHTH OF AN INCH, EXCEPT STRUCTURAL STEEL DIMENSIONS WHICH ARE TO THE NEAREST ONE-SIXTEENTH OF AN INCH.
3. ALL ELEVATIONS ARE REFERENCED TO THE NATIONAL GEODETIC VERTICAL DATUM OF 1988 (NGVD 88).
4. COORDINATES USED ON THESE PLANS ARE BASED ON THE STATEWIDE COORDINATE SYSTEM, THE NORTH AMERICAN DATUM OF 1983 (NAD 83).
5. ANGLES ARE SHOWN TO THE NEAREST SECOND.
6. ALL FOOTINGS SHALL BE APPROVED BY THE ENGINEER AS TO DIMENSIONS, ELEVATIONS, AND SUITABILITY OF FOUNDATION MATERIAL BEFORE THE PLACING OF CONCRETE.
7. ALL WALLS ARE DRAWN LOOKING AT THE EXPOSED FACES.
8. THE EXISTING UTILITIES SHOWN ON THE PLANS ARE APPROXIMATE AND WERE LOCATED USING THE BEST AVAILABLE INFORMATION. NO BUILDING SERVICE CONNECTIONS (ELECTRIC, TELEPHONE, GAS, WATER, SANITARY AND OTHERS) ARE SHOWN. THE CONTRACTOR IS TO ASSUME THAT SERVICES TO ALL BUILDINGS ARE PRESENT.
9. BOTH FEDERAL AND STATE LAW (RI. GENERAL LAW 39-1.2) REQUIRE NOTIFICATION OF APPROPRIATE UTILITY COMPANIES BEFORE DIGGING, TRENCHING, BLASTING, DEMOLISHING, BORING, BACK FILLING, GRADING, LANDFILLING, OR OTHER EARTH MOVING OPERATIONS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY ALL UTILITY COMPANIES (INCLUDING THROUGH THE "DIG SAFE" PROGRAM) TO ENSURE THAT ALL UTILITIES, BOTH UNDERGROUND AND OVERHEAD, HAVE BEEN MARKED BEFORE COMMENCEMENT OF SUCH WORK. THE CONTRACTOR SHOULD UNDERSTAND THAT NOT ALL UTILITIES SUBSCRIBE TO THE "DIG SAFE" PROGRAM. ANY DAMAGE TO EXISTING UTILITIES MARKED IN THE FIELD, OR AS A RESULT OF FAILING TO CONTACT THE APPROPRIATE UTILITY COMPANIES, SHALL BE REPAIRED OR REPLACED (AS DEEMED APPROPRIATE BY THE STATE AND/OR THE IMPACTED UTILITY COMPANY) AT NO ADDITIONAL COST TO THE STATE.
10. ALL ANCHOR BOLTS SHALL BE ASTM DESIGNATION A 307 AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO DESIGNATION M 232 OR METALIZED IN ACCORDANCE WITH SECTION M.05. SWAGED RODS SHALL BE AASHTO DESIGNATION M 270 GRADE 36 AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO DESIGNATION M 232.
11. ALL ANCHOR BOLTS SHALL BE SET PRIOR TO PLACEMENT OF CONCRETE UNLESS OTHERWISE AUTHORIZED BY THE ENGINEER.
12. CARBON STEEL BOLTS SHALL CONFORM TO ASTM A307.
13. ALL WORK SHALL BE COMPLETED IN THE DRY.

1. DESIGN SPECIFICATIONS
 - THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8th EDITION, NOVEMBER 2017, INCLUDING ALL INTERIM REVISIONS TO DATE.
 - THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL 2007 EDITION INCLUDING ALL REVISIONS TO DATE.
 - THE 2018 REVISION OF AND SUPPLEMENTS TO THE RHODE ISLAND DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION (RI STANDARD SPECIFICATIONS).
 - IN CASE OF CONFLICT, THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL SHALL GOVERN.
2. LOAD MODIFIERS

THE LOAD MODIFIERS FOR THIS PROJECT ARE AS FOLLOWS:

 - THE LOAD MODIFIER FOR DUCTILITY SHALL BE TAKEN AS 1.0 FOR ALL LIMIT STATES.
 - THE LOAD MODIFIER FOR REDUNDANCY SHALL BE TAKEN AS 1.00.
 - THE LOAD MODIFIER FOR OPERATIONAL IMPORTANCE SHALL BE TAKEN AS 1.05.
3. LIVE LOADS
 - THE DESIGN LIVE LOAD SURCHARGE SHALL CONFORM TO AASHTO SECTION 3.11.6
4. FOUNDATION DESIGN DATA

DEEP FOUNDATIONS

 - THE FACTORED AXIAL AND UPLIFT RESISTANCES FOR THE DEEP FOUNDATION ARE AS FOLLOWS:

FACTORED AXIAL RESISTANCE (KIPS)			
GEOTECHNICAL		STRUCTURAL	
STRENGTH LIMIT STATE	EXTREME LIMIT STATE	STRENGTH LIMIT STATE	EXTREME LIMIT STATE
28.42	4.67	107.53	73.11

1. ALL TIMBER SHALL BE SAWN LUMBER, SURFACED FOUR SIDES (242) UNLESS OTHERWISE NOTED, AND SHALL COMPLY WITH THE REQUIREMENTS OF AASHTO M168 AND THE SPECIFICATIONS.
2. SAWN LUMBER SHALL BE PRESSURE TREATED WITH CCA CONFORMING TO AWPA STANDARD P5 AND AITC 109 TO A MINIMUM NET RETENTION OF 0.40 LB./CF IN ACCORDANCE WITH AWPA STANDARD. ALL MEMBERS SHALL BE FABRICATED BEFORE TREATMENT AND DRIED TO A MOISTURE CONTENT OF 19 PERCENT OR LESS AFTER TREATMENT.
3. ALL TIMBER SHALL BE PREDRILLED PRIOR TO TREATMENT
4. ALL TIMBER SHALL BE PRECISION END TRIMMED TO LENGTH WITH $\frac{1}{4}$ " UNDER LENGTH AND NO OVER LENGTH TOLERANCE PERMITTED.
5. ALL TREATED TIMBER THAT IS FIELD CUT, BORED THRU, DRILLED INTO OR DAMAGED SHALL BE TREATED AS OUTLINED IN AWPA STANDARD M4 WHICH REQUIRES THAT ALL CUTS, HOLES, OR INJURIES TO TREATED WOOD BE PROTECTED BY BRUSHING, SPRAYING, DIPPING, OR SOAKING IN AN APPROVED PRESERVATIVE.
6. CONSTRUCTION REQUIREMENTS SHALL CONFORM TO STATE SPECIFICATIONS. ALL TIMBER SHALL BE CUT TO LENGTH AND DRESSED TO SIZE REQUIRED PRIOR TO TREATMENT.
7. TIMBER WHICH AT THE DISCRETION OF THE ENGINEER IS SEVERELY WARPED, BOWED, SPLIT, OR SPLINTERED SHALL NOT BE INCORPORATED IN THE WORK.

- AASHTO DESIGNATION M 31, GRADE 60

- CLASS XX 3/4" $f'_c=4,000$ PSI

1. CLASSES OF CONCRETE SHALL BE CLASS XX AS DESCRIBED IN THE RI STANDARD SPECIFICATIONS AND THE SPECIAL PROVISIONS OF THE SPECIFICATIONS. REFER TO THE "MATERIAL" NOTES FOR CLASSES OF CONCRETE SPECIFIED FOR VARIOUS COMPONENTS.

2. THE CONTRACTOR MAY, AT THE APPROVAL OF THE ENGINEER, PROPOSE THE USE OF SELF-CONSOLIDATING CONCRETE FOR CLASS HP OR XX ON THIS PROJECT. SECTION 606 "SELF CONSOLIDATING CONCRETE (SCC)", CONTAINS THE REQUIREMENTS FOR MODIFYING ALL CLASSES OF CONCRETE MIX DESIGN FOR SELF-CONSOLIDATING APPLICATIONS.

3. ALL PORTLAND CEMENT CONCRETE SHALL BE AIR-ENTRAINED.

4. ALL REINFORCING STEEL SHALL BE GALVANIZED. ALL WIRE TIES AND MISCELLANEOUS HARDWARE USED FOR PLACEMENT OF GALVANIZED REINFORCING SHALL ALSO BE GALVANIZED. GALVANIZED COATING FOR REINFORCING STEEL SHALL CONFORM TO ASTM A767 CLASS 1.

5. ALL CRITICAL LAP SPICES SHALL BE AS SHOWN ON THE PLANS. ALL SPICES NOT SHOWN ON THE PLANS SHALL BE LAPPED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR CLASS B LAP SPICES.

6. UNLESS OTHERWISE INDICATED ON THE PLANS, ALL MAIN REINFORCING BARS SHALL HAVE THE FOLLOWING MINIMUM COVER:

CONCRETE CAST AGAINST OR PERMANENTLY EXPOSED TO EARTH (FOOTINGS)	3"
ALL OTHER BARS	2"

COVER TO TIES AND STIRRUPS MAY BE 0.5 INCH LESS THAN THE ABOVE VALUES SPECIFIED FOR MAIN REINFORCING, BUT IN NO CASE LESS THAN 1.5 INCHES.

7. HORIZONTAL CONSTRUCTION JOINTS OTHER THAN THOSE SHOWN ON PLANS WILL NOT BE PERMITTED WITHOUT A WRITTEN REQUEST BY THE CONTRACTOR AND PRIOR AUTHORIZATION BY THE ENGINEER.

8. ALL EXPOSED EDGES AND REENRANT CORNERS NOT OTHERWISE DETAILED ON THE PLANS SHALL HAVE A MINIMUM 3/4" CHAMFER.

9. ALL JOINT SEALANT SHALL BE POLYURETHANE, POLYURETHANE ELASTOMERIC, OR SILICONE SEALANT AS DESIGNATED ON THE PLANS. THE COLOR OF THE JOINT SEALANT, WHERE EXPOSED, SHALL BE NEUTRAL (LIGHT GRAY OR TAN). THE COLOR OF THE SEALANT, WHERE NOT EXPOSED, WILL BE AT THE DISCRETION OF THE CONTRACTOR.

10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PREVENTING CONCRETE STAINS OR DISCOLORATIONS DURING CONSTRUCTION UNTIL SUCH TIME WHEN THE SURFACES ARE APPROVED AND ACCEPTED. ANY CONCRETE STAINS OR DISCOLORATIONS OCCURRING PRIOR TO ACCEPTANCE OF THE SURFACES SHALL BE REMOVED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE STATE.

11. UNLESS OTHERWISE NOTED ON THE PLANS, JOINT FILLER IS TO BE A PREFORMED, NON-EXPANSIVE, NON-EXTRUDING TYPE IN ACCORDANCE WITH SECTION M.02.11.1 OF THE RI STANDARD SPECIFICATIONS.

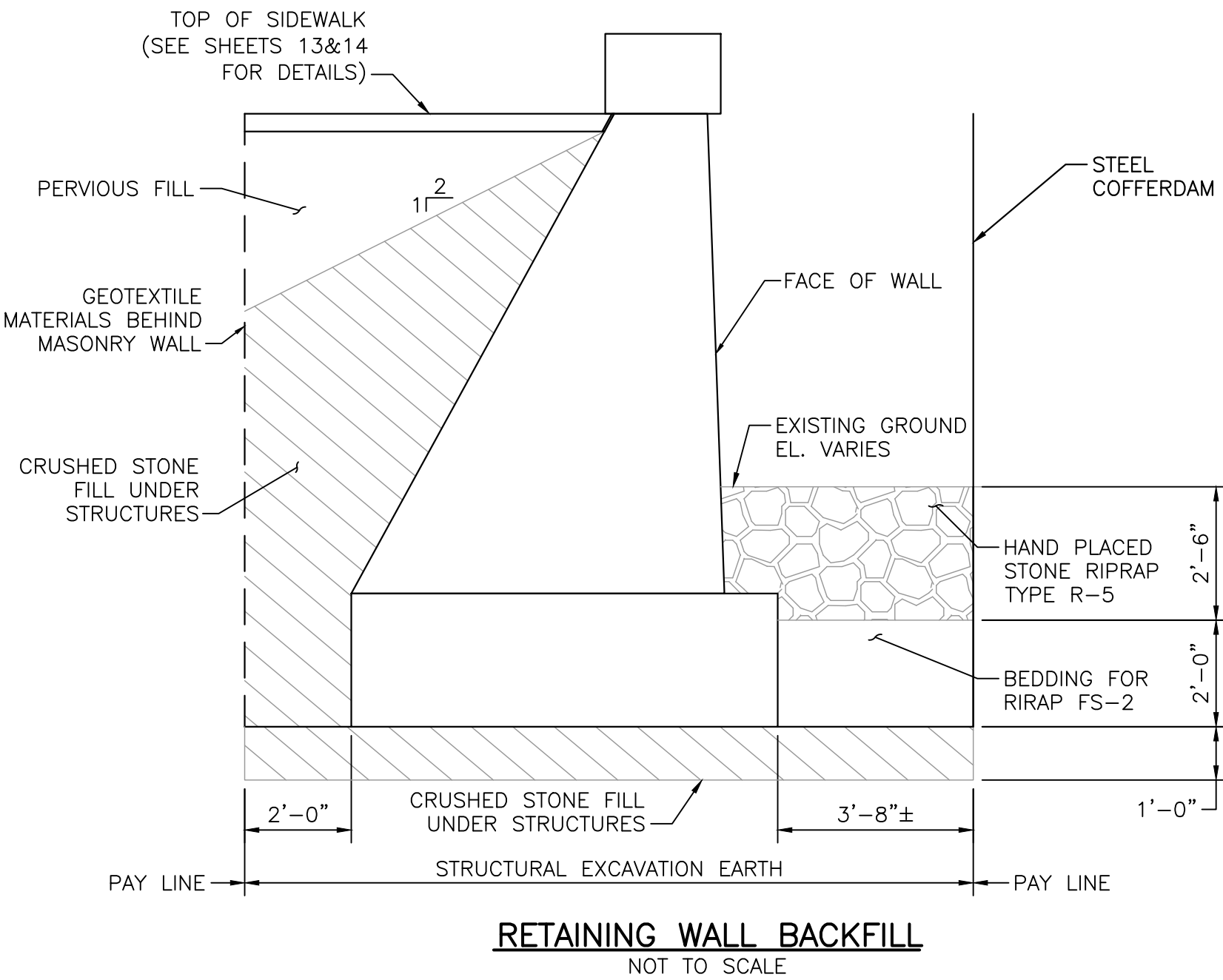
12. EMBEDMENT LENGTHS FOR DRILLED AND GROUTED DOWELS SHALL BE IN ACCORDANCE WITH SECTION 819 OF THE RI STANDARD SPECIFICATIONS.

13. IN ACCORDANCE WITH THE RI STANDARD SPECIFICATIONS, ALL METAL TIES, NON-METALLIC TIES OR ANCHORAGES WHICH ARE REQUIRED FOR CONCRETE FORMWORK SHALL BE SO CONSTRUCTED THAT THEY CAN BE REMOVED TO AT LEAST ONE INCH BELOW THE EXPOSED SURFACE OF THE CONCRETE WITHOUT CAUSING DAMAGE TO THE CONCRETE SURFACE. SNAP TIES MAY BE USED ONLY IF APPROVED BY THE ENGINEER. IF THE CONTRACTOR PROPOSES TO USE THEM, A CATALOG CUT AND OTHER NECESSARY INFORMATION MUST BE SUBMITTED TO THE ENGINEER TO DEMONSTRATE THAT THE TIES WILL SNAP-OFF FAR ENOUGH INTO THE CONCRETE TO ALLOW FOR PROPER PATCHING. SNAP TIES MUST PROVIDE ADEQUATE STRENGTH TO SUPPORT THE FORMS. ALL CAVITIES SHALL BE FILLED WITH AN APPROVED CEMENT MORTAR MEETING THE REQUIREMENTS OF ASTM C 928.

OBSERVED HIGH WATER ELEVATION	=	3.75
OBSERVED LOW WATER ELEVATION	=	1.50

NOAA MEAN HIGHER HIGH WATER ELEVATION	=	1.75
NOAA MEAN HIGH WATER ELEVATION	=	1.49
NOAA MEAN LOW WATER ELEVATION	=	-2.20
NOAA MEAN LOWER LOW WATER ELEVATION	=	-2.37

ACTUAL WATER ELEVATIONS WERE OBSERVED TO BE HIGHER THAN NOAA ELEVATIONS.
THE CONTRACTOR SHALL NOTE THAT HIGHER AND LOWER TIDES ARE POSSIBLE.



www.BETA-Inc.com

No. **ODD.M. WARZECKI**
 No. **9754**
 No. **9414**
 REGISTERED
 PROFESSIONAL ENGINEER
 CIVIL-17-21

PROJECT

Wickford, RI

STRUCTURAL GENERAL NOTES

NO.	REVISIONS	DATE
DRAWN BY:	BN	
DESIGNED BY:	JR	
CHECKED BY:	TW	
ISSUE DATE:	4/17/2024	
BETA JOB NO.:	7038	

AS SHOWN

UNLESS OTHERWISE NOTED OR CHANGED BY REPRODUCTION

SHEET NO.

PILE CUTOFF
EL. -4.50
B.O. FOOTING
EL. -5.50

B.O. PILE TIP
EL. -24.50±

Geologic - Earth Exploration, Inc.										CLIENT: BETA		BORING #:					
7 Sherwood Drive TEL 5083844434										Norfolk, MA 02056 FAX		WB-1					
PROJECT: Wickford Waterfront Improvements										LOCATION: North Kingstown, RI		PAGE					
1 OF 1																	
File #: 22127										CASING		SAMPLER CORE BARREL		Surface Elevation:			
Date Started: 8/17/22										TYPE HW		SS NX		Station:			
Date Completed: 8/17/22										SIZE 4"		2"		Groundwater level readings			
Driller: J. Martinelli										HAMMER 300#		140#		Date _____ Depth _____			
Site Rep: B. Nelson										FALL 30"		30"		Date _____ Depth _____			
Depth ft										Sample		Strata Change ft		Sample Description			
S-1 0.0-2.0										24 12		11-13-6-5		S-1 Dry medium dense dark brown SILT and medium SAND, some Gravel			
S-2 4.0-6.0										24 12		10-5-7-7		S-2 Moist medium dense dark brown SILT and coarse SAND, trace Clay			
S-3 9.0-11.0										24 12		5-1-1-2		S-3 Moist loose dark brown SILT and medium SAND			
S-4 14.0-16.0										24 10		6-6-9-14		S-4 Wet medium dense brown/gray SILT and medium SAND			
S-5 19.0-21.0										24 16		9-13-14-13		S-5 Similar to S-4			
S-6 24.0-26.0										24 24		7-9-9-18		S-6 Moist medium dense brown SILT and fine SAND			
S-7 29.0-31.0										24		13-23-11-13		S-7 Wet dense gray SILT			
S-8 34.0-34.1										1 0		120/1"		S-8 No recovery Possible top of rock at 34.1', roller bit from 34.1' to 36' _____			
C1 36.0-41.0										60 60				C1 ROD= 71%			
C2 41.0-46.0										60 60				C2 ROD= 100%			
												46.0		Bottom of exploration at 46.0'			
Ground Surface to _____ used _____ then _____																	
Proportions Used										Cohesive Consistency		Cohesionless Density		Sample Type			
Trace 0 to 10%										0-2 Very Soft		9-15 Stiff		0-10 Loose		UP = Fixed Piston	
Little 10 to 20%										3-4 Soft		16-30 V-Stiff		10-30 M-Dense		UT = Shelby Tube	
Some 20 to 35%										5-8 M-Stiff		31+ Hard		30-50 Dense		OE = Open End Rod	
And 35 to 50%														50+ V-Dense		* = 300# hammer	
Notes:										1. The stratification lines represent the approximate boundary between soil types. The transition may be gradual.							
										2. Water level readings were made in the drill hole during or at the completion of drilling. The water level may fluctuate over time.							
Remarks:																	

PILE CUTOFF
EL. -4.50
B.O. FOOTING
EL. -5.50

B.O. PILE TIP
EL. -24.50±

Geologic - Earth Exploration, Inc.										CLIENT: BETA		BORING #:					
7 Sherwood Drive TEL 5083844434										Norfolk, MA 02056 FAX		WB-2					
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Site Rep: B. Nelson										FALL 30"		30"		Date _____ Depth _____			
Depth ft										Sample		Strata Change ft		Sample Description			
S-1 0.0-2.0										24 8		10-16-12-11		S-1 Dry medium dense dark brown SILT, medium SAND and coarse GRAVEL			
S-2 4.0-6.0										24 1		4-8-10-9		S-2 Moist medium dense brown coarse SAND and GRAVEL			
S-3 9.0-11.0										24 10		8-1-2-1		S-3 Moist loose brown fine to coarse SAND, some Silt			
S-4 14.0-16.0										24 15		WOH		S-4 Moist loose dark gray SILT, trace Clay and Organic			
S-5 19.0-21.0										24 10		WOH-12-7-9		S-5 Wet medium dense dark gray SILT and fine SAND			
S-6 24.0-25.7										20 8		23-59-44 -100/2		S-6 Wet very dense gray SILT, SAND and coarse GRAVEL Possible top of rock at 26', roller bit to 29'			
												26.0					
												29.0		Bottom of exploration at 29.0'			
Ground Surface to _____ used _____ then _____																	
Proportions Used										Cohesive Consistency		Cohesionless Density		Sample Type			
Trace 0 to 10%										0-2 Very Soft		9-15 Stiff		0-10 Loose		UP = Fixed Piston	
Little 10 to 20%										3-4 Soft		16-30 V-Stiff		10-30 M-Dense		UT = Shelby Tube	
Some 20 to 35%										5-8 M-Stiff		31+ Hard		30-50 Dense		OE = Open End Rod	
And 35 to 50%														50+ V-Dense		* = 300# hammer	
Notes:										1. The stratification lines represent the approximate boundary between soil types. The transition may be gradual.							
										2. Water level readings were made in the drill hole during or at the completion of drilling. The water level may fluctuate over time.							
Remarks:																	

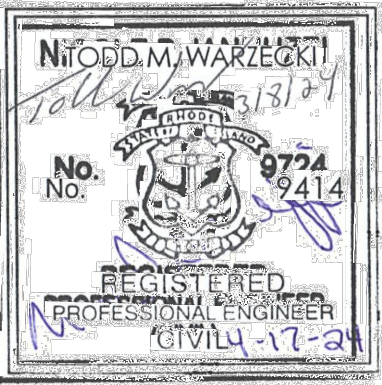
BORING NOTES:

1. LOCATION OF BORINGS SHOWN ON THE PLAN THUS: WB-1
2. BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
3. FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 1 1/8" I.D. SPLIT SPOON SAMPLER 6" USING A 140 POUND WEIGHT FALLING 30".
4. ALL BORINGS WERE MADE IN AUGUST, 2022.
5. BORINGS WERE MADE BY GEOLOGIC - EARTH EXPLORATION, INC., 7 SHERWOOD DRIVE, NORFOLK, MA 02056.
6. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.

PREPARED BY



REGISTERED PROFESSIONAL



SUBCONSULTANT

PROJECT

Wickford Waterfront
Improvements

Wickford, RI

TITLE

BORING LOGS

NO.	REVISIONS	DATE
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DRAWN BY: BN
DESIGNED BY: JR
CHECKED BY: TW
ISSUE DATE: 4/17/2024
BETA JOB NO.: 7038

SCALE

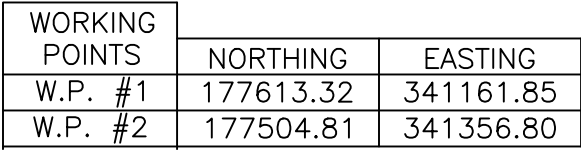
AS SHOWN

UNLESS OTHERWISE NOTED OR CHANGED BY REPRODUCTION

SHEET NO.

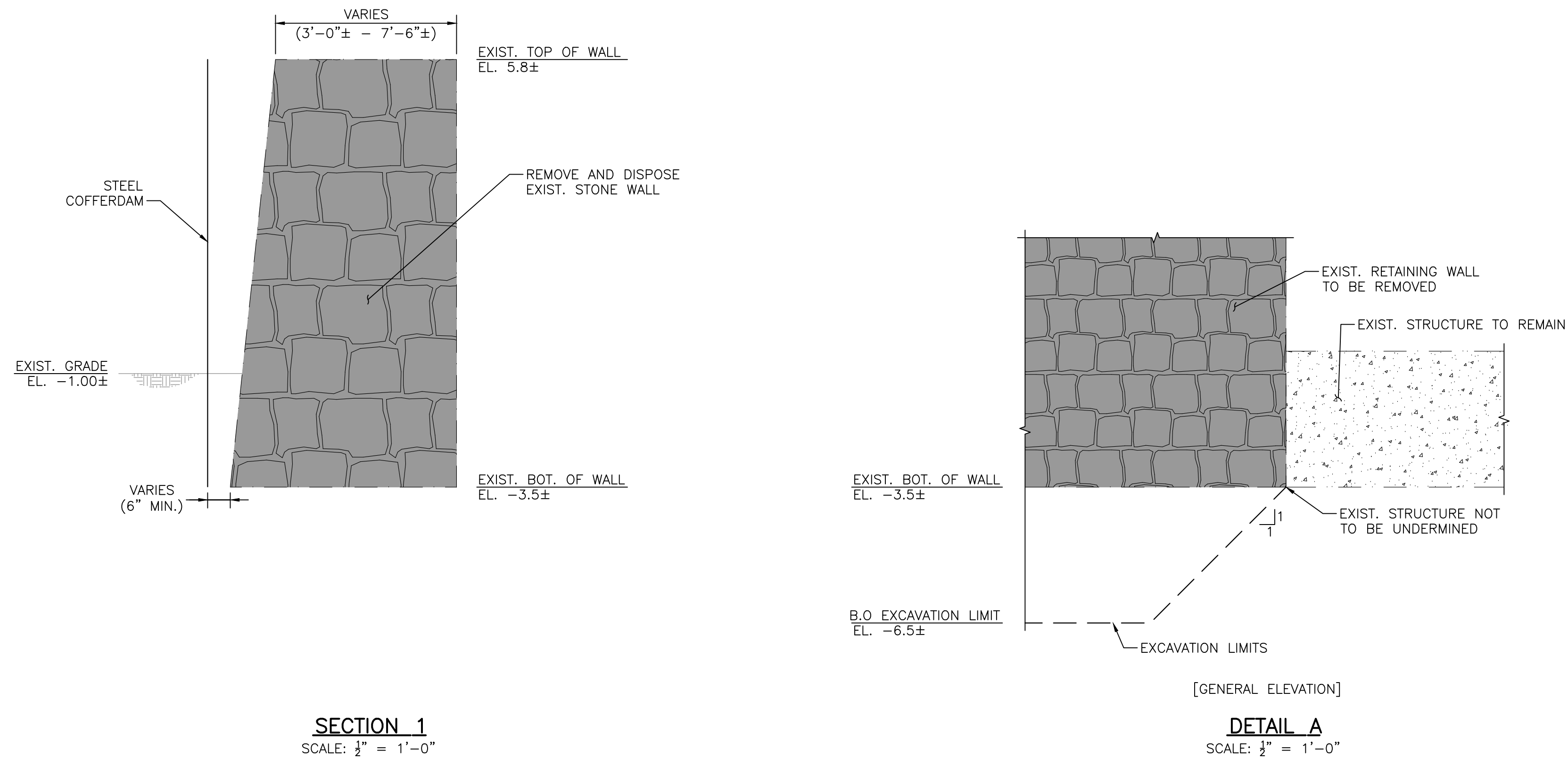
23

NO.	REVISIONS	DATE
DRAWN BY:	BN	
DESIGNED BY:	JR	
CHECKED BY:	TW	
ISSUE DATE:	4/17/2024	
BETA JOB NO.:	7038	



DEMOLITION PLAN
SCALE: $\frac{1}{8}" = 1'-0"$

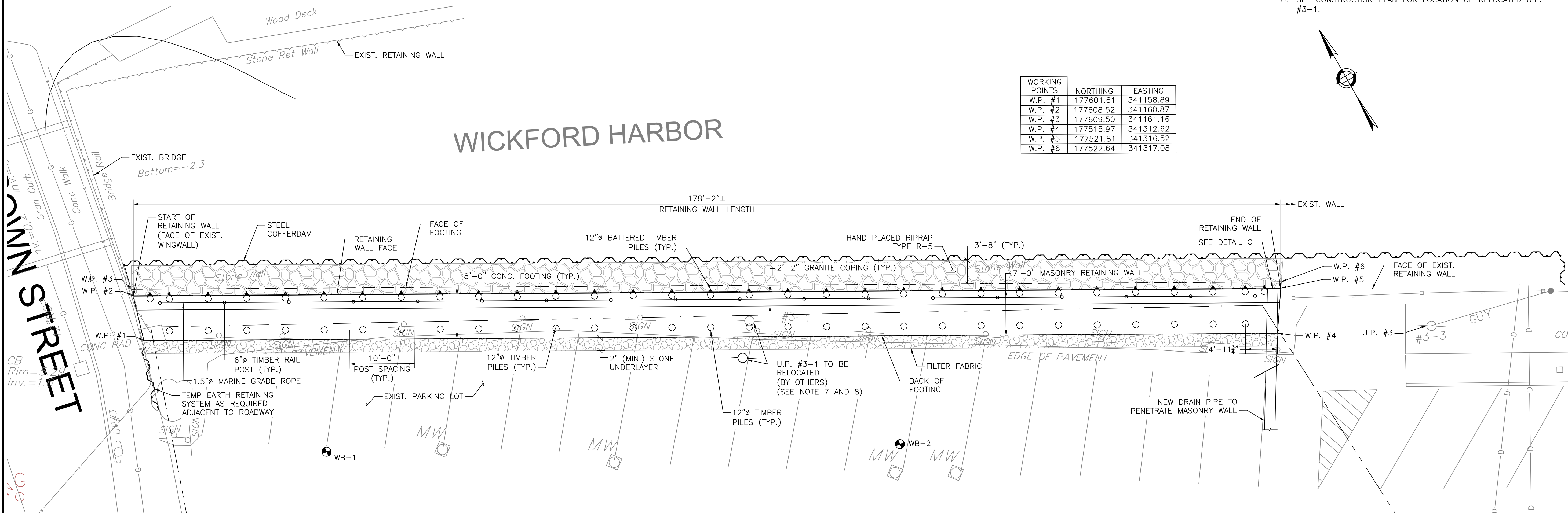
1.  INDICATES AREA TO BE DEMOLISHED.
2. THESE DRAWINGS ARE FOR CONCEPTUAL PURPOSES ONLY. ACTUAL CONFIGURATION OF EXISTING WALL MAY VARY.
3. LIMITS OF STRUCTURAL EXCAVATION MASONRY COVERED UNDER PAY ITEM 203.0310.
4. EXIST. U.P. TO BE RELOCATED BY RHODE ISLAND ENERGY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE WORK & SCHEDULE WITH UTILITY OWNER SO THAT CONSTRUCTION OPERATIONS ARE NOT INTERRUPTED.
5. BOTTOM ELEVATION OF EXISTING WALLS ARE UNKNOWN. IF EXCAVATION WILL RESULT IN UNDERMINING EXISTING WALLS TO REMAIN, LIMIT OF EXCAVATION SHALL BE IN ACCORDANCE WITH DETAIL A.
6. REMOVAL OF EXISTING STONE RUBBLE RETAINING WALL TO THE LIMITS SHOWN ON THIS DRAWING SHALL BE PAID UNDER ITEM 02202 - STRUCTURAL EXCAVATION MASONRY.



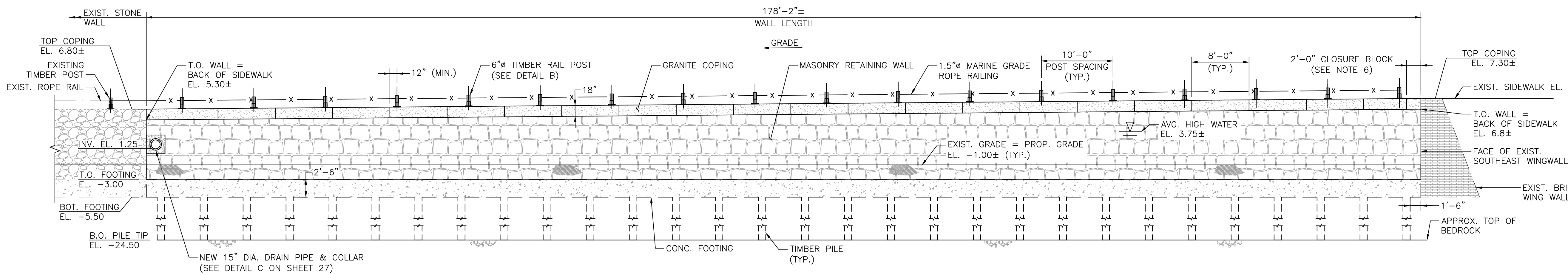
SECTION 1
SCALE: $\frac{1}{2}" = 1'-0"$

DETAIL A
SCALE: $\frac{1}{2}" = 1'-0"$

4/16/2024 4:13 PM G:\PLANNING LANDSCAPE\700S7038 - WICKFORD WATERFRONT IMPROVEMENTS\STRUCTURAL\DRAWING FILES\PLANSET\7038_SR(4).DWG (BETA STD BW.CTB)



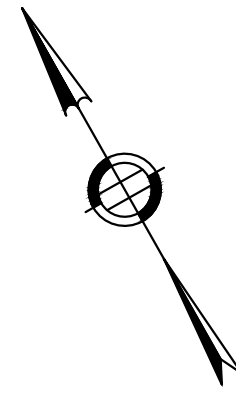
GENERAL PLAN
SCALE: 1/8" = 1'-0"



[LOOKING SOUTH]
GENERAL ELEVATION
SCALE: 1/8" = 1'-0"

NOTES:

1. THE CONTRACTOR MUST NOTIFY DIG SAFE PRIOR TO ANY EXCAVATION OR DEMOLITION WORK IN PUBLIC OR PRIVATE WAYS OR UTILITY COMPANY RIGHT-OF-WAY EASEMENT.
2. AREAS OUTSIDE THE LIMITS OF PROPOSED WORK DISTURBED BY THE CONTRACTOR'S OPERATIONS SHALL BE RESTORED BY THE CONTRACTOR TO THEIR ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE.
3. JOINTS BETWEEN NEW HOT MIX ASPHALT PAVEMENT AND SAWCUT EXISTING PAVEMENT SHALL BE SEALED WITH BITUMEN AND BACKSEALED.
4. PILE DIMENSIONS NOT SHOWN FOR CLARITY SEE FOOTING AND PILE LAYOUT PLAN ON SHEET 26.
5. NEW STONE SHALL MATCH EXISTING IN SIZE, SHAPE, AND COLOR.
6. MASON TO CUT GRANITE BLOCK EVEN AND SQUARE. BLOCK SHOULD BE UNIFORM AND CONSISTENT WITH THE REST OF THE COPING WITH NO VISIBLE GAP PRESENT.
7. IT IS THE CONTRACTORS RESPONSIBILITY TO COORDINATE RELOCATION OF U.P. #3-1 WITH RHODE ISLAND ENERGY.
8. SEE CONSTRUCTION PLAN FOR LOCATION OF RELOCATED U.P. #3-1.

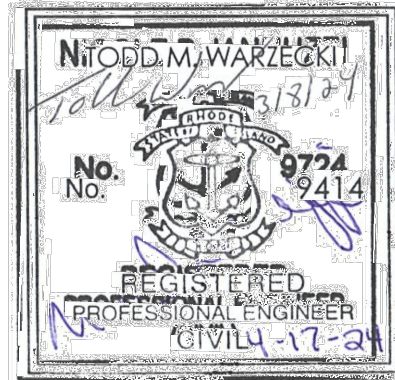


WORKING POINTS	NORTHING	EASTING
W.P. #1	177601.61	341158.89
W.P. #2	177608.52	341160.87
W.P. #3	177609.50	341161.16
W.P. #4	177515.97	341312.62
W.P. #5	177521.81	341316.52
W.P. #6	177522.64	341317.08

PREPARED BY



REGISTERED PROFESSIONAL



SUBCONSULTANT

PROJECT

Wickford Waterfront Improvements

Wickford, RI

TITLE

WALL PLAN & ELEVATION

NO.	REVISIONS	DATE
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DRAWN BY:	BN
DESIGNED BY:	JR
CHECKED BY:	TW
ISSUE DATE:	4/17/2024
BETA JOB NO.:	7038

SCALE

AS SHOWN

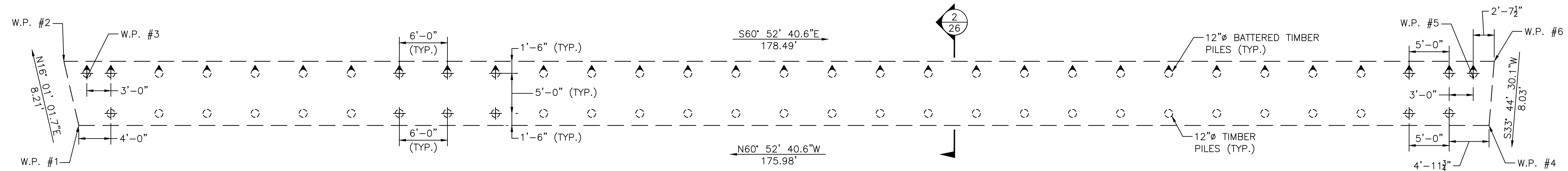
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SHEET NO.

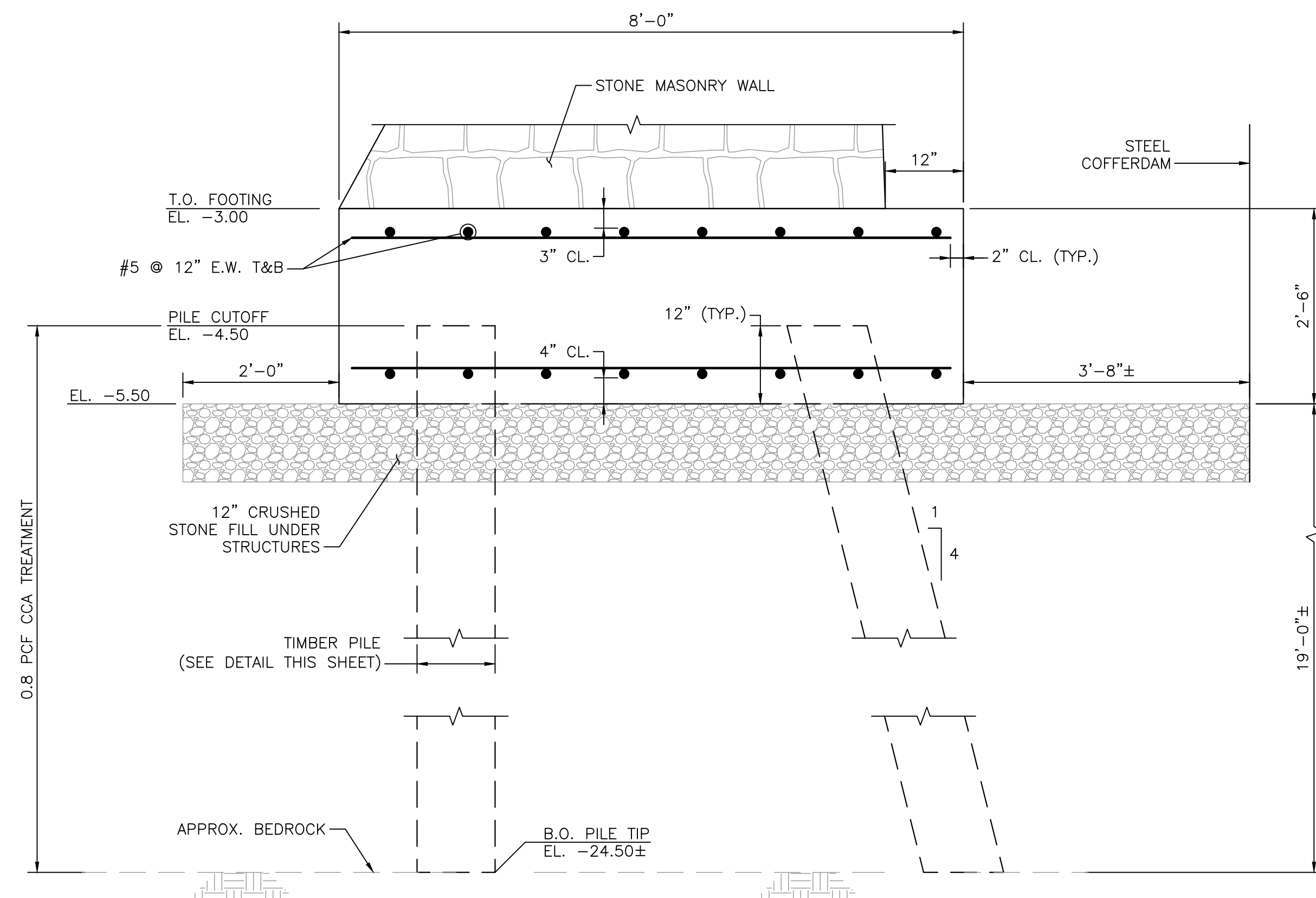
[illegible]

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BETA JOB NO.:	7038	

WORKING POINTS	NORTHING	EASTING	
	W.P. #1	177601.61	341158.89
	W.P. #2	177606.36	341162.68
	W.P. #3	177609.50	341161.16
	W.P. #4	177515.97	341312.62
	W.P. #5	177522.17	341313.82
	W.P. #6	177522.64	341317.08



7. DETERMINATION OF THE DRIVEN PILE RESISTANCE SHALL BE PERFORMED USING THE AXIAL/COMPRESSION LOAD TEST. THE TEST SHALL BE PERFORMED ON THE FIRST PILE INSTALLED PRIOR TO INSTALLING THE REMAINING PRODUCTION PILES.



12"Ø 0.8 PCF CCA TREATED
CLASS A TIMBER PILE

TIMBER PILE DETAIL
SCALE: $1\frac{1}{2}" = 1'-0"$

